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Biological Wastewater Treatment II – MBBR Processes

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Biological Wastewater Treatment II – MBBR Processes

Harlan H. Bengtson, PhD, P.E.

COURSE CONTENT

1. Introduction

Biological wastewater treatment is very widely used for removal of biodegradable materials from wastewater. The first course in this sequence, Biological Wastewater Treatment I – Activated Sludge, starts with a discussion of the biochemical oxygen demand that is created by biodegradable materials in water and the reason why such materials must be removed from wastewater. This course is about the Moving Bed Biofilm Reactor (MBBR) wastewater treatment process, including background information about the process and a description of the process, as well as process design calculations for several different configurations of the MBBR process and numerous example calculations.

2. Learning Objectives

At the conclusion of this course, the student will

- Know the differences between attached growth and suspended growth biological wastewater treatment processes
- Be familiar with the components and general configuration of an MBBR wastewater treatment process
- Be able to calculate the loading rate of a wastewater constituent to an MBBR process (in lb/day and g/day) for a specified wastewater flow rate and constituent concentration
- Be able to calculate the required carrier surface area for an MBBR wastewater treatment process for a specified SALR and loading rate
- Be able to calculate the required MBBR tank volume for specified carrier surface area, carrier specific surface area, and the carrier fill %.

- Be able to calculate the liquid volume in an MBBR tank for known tank volume, carrier volume and carrier % void space
- Be able to calculate the BOD, NH₃-N, or NO₃-N removal rate for known values of the surface area removal rate (SARR) and design carrier surface area
- Be able to calculate an estimated effluent BOD, NH₃-N, or NO₃-N concentration based on known values of the appropriate loading rate, estimated removal rate, and design wastewater flow rate
- Be able to make process design calculations for a post-Anoxic denitrification MBBR process, including required tank sizes, estimated effluent concentrations, alkalinity requirement and carbon source requirement
- Be able to make process design calculations for a pre-anoxic denitrification MBBR process, including required tank sizes, estimated effluent concentrations, and alkalinity requirements

3. Topics Covered in this Course

- I. Description of the MBBR (Moving Bed Biofilm Reactor) Process
- II. Single Stage BOD Removal MBBR Process Design Calculations
- III. Two-Stage BOD Removal MBBR Process Design Calculations
- IV. Single Stage Nitrification MBBR Process Design Calculations
- V. Two-Stage BOD Removal and Nitrification MBBR Process Design Calculations
- VI. Denitrification Background Information
- VII. Post-Anoxic Denitrification MBBR Process Design Calculations
- VIII. Pre-Anoxic Denitrification MBBR Process Design Calculations

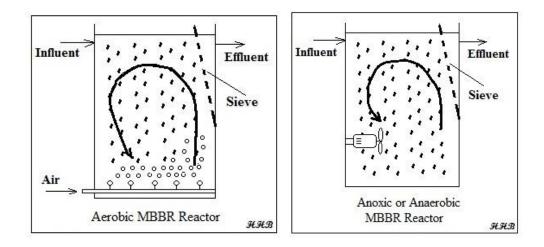
4. Description of the MBBR (Moving Bed Biofilm Reactor) Process

Initial Development of the MBBR Process: The MBBR process for wastewater treatment was invented and initially developed by Professor Hallvard Ødegaard in the late 1980s at the Norwegian University of Science and Technology. Use of this wastewater treatment process has spread rapidly. Per Ødegaard, 2014 (Reference #1 at the end of this course), there were already more than 800 MBBR wastewater treatment plants in more than 50 countries in 2014, with about half treating domestic wastewater and about half treating industrial wastewater. At least part of the reason for the interest in the MBBR process is its small footprint in comparison with other biological treatment processes. The tank volume needed for a MBBR process is typically significantly less than that needed for either an activated sludge process or a trickling filter designed to treat the same wastewater flow.

General Description of the MBBR Process: The MBBR process is an attached growth biological wastewater treatment process. That is, the microorganisms that carry out the treatment are attached to a solid medium, as in trickling filter or RBC systems. By contrast, in a suspended growth biological wastewater treatment process, like the activated sludge process, the microorganisms that carry out the treatment are kept suspended in the mixed liquor in the aeration tank.

In the conventional attached growth biological treatment processes, like trickling filter or RBC systems, the microorganisms are attached to a medium that is fixed in place and the wastewater being treated flows past the surfaces of the medium with their attached biological growth. In contrast, an MBBR process utilizes small plastic carrier media (described in more detail in the next section) upon which the microorganisms are attached. The MBBR treatment processes typically take place in a tank similar to an activated sludge aeration tank. The carrier media are kept suspended by a diffused air aeration system for an aerobic process or by a mechanical mixing system for an anoxic or anaerobic process, as illustrated in the figures below. A sieve is typically used at the MBBR tank exit to keep the carrier media in the tank.

Primary clarification is typically used ahead of the MBBR tank. Secondary clarification is also typically used, but there is no recycle activated sludge sent back into the process, because an adequate microorganism population is maintained attached to the media.



The MBBR Media Support Carrier System: MBBR processes use plastic media support carriers similar to those shown in the figure below. As shown in that figure, the carrier is typically designed to have a high surface area per unit volume, so that there is a lot of surface area on which the microorganisms attach and grow. Media support carriers like those shown in the figure are available from numerous vendors. Two properties of the carrier are needed for the process design calculations to be described and discussed in this course. Those properties are the specific surface area in m²/m³ and the void ratio. The specific surface area of MBBR carriers is typically in the range from 350 to 1200 m²/m³ and the void ratio typically ranges from 60% to 90%. Design values for these carrier properties should be obtained from the carrier manufacturer or vendor.

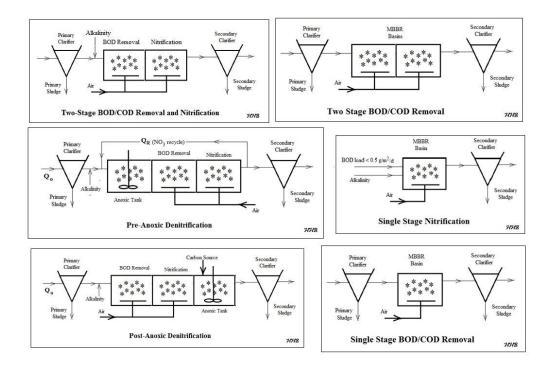


Typical MBBR Media Support Carriers

MBBR Wastewater Treatment Process Alternatives: The MBBR wastewater treatment process is quite flexible and can be used in several different ways. The figure below shows flow diagrams for the following six alternatives. Note that, as previously

mentioned, primary clarification and secondary clarification are shown for all of the process alternatives, but there is no sludge recycle as in a conventional activated sludge process. Also, note that a clarifier is not typically used between stages in a two or three-stage MBBR process.

- 1. Single stage BOD removal
- 2. Two stage BOD removal
- 3. Two stage BOD removal and Nitrification
- 4. Single stage tertiary Nitrification
- 5. Pre-Anoxic Denitrification
- 6. Post-Anoxic Denitrification



Overview of MBBR Process Design Calculations: The key empirical design parameter used to determine the required MBBR tank size is the surface area loading rate (SALR) in g/m²/d. The g/d in the SALR units refers to the g/d of the parameter being removed and the m² in the SALR units refers to the surface area of the carrier. Thus, for BOD removal the SALR would be g BOD/day entering the MBBR tank per m² of carrier surface area. For a nitrification reactor, the SALR would be g NH₃-N/day entering the MBBR tank per m² of carrier surface area. Finally, for denitrification design, the SALR would be g NO₃-N/day per m² of carrier surface area.

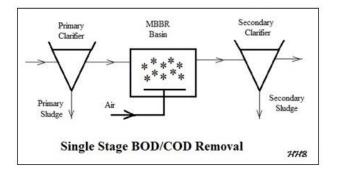
For any of these processes, a design value for SALR can be used together with design values of wastewater flow rate and BOD, ammonia or nitrate concentration, to calculate the required carrier surface area in the MBBR tank. The design carrier volume can then

be calculated using a known value for the carrier specific surface area (m^2/m^3) . Finally, a design value for the carrier fill % can be used to calculate the required tank volume.

Process design calculations for each of the process alternatives shown in the figure above will be covered in the next several sections.

5. Single Stage BOD Removal MBBR Process Design Calculations

An MBBR single stage BOD removal process may be used as a free-standing secondary treatment process or as a roughing treatment prior to another secondary treatment process, in some cases to relieve overloading of an existing secondary treatment process. In either case the key design parameter for sizing the MBBR tank is the surface area loading rate (SALR), typically with units of g/m²/day, that is g/day of BOD coming into the MBBR tank per m² of carrier surface area. Using design values for wastewater flow rate and BOD concentration entering the MBBR tank, the loading rate in g BOD/day can be calculated. Then dividing BOD loading rate in g/day by the SALR in g/m²/day gives the required carrier surface area in m². The carrier fill %, carrier specific surface area, and carrier % void space can then be used to calculate the required carrier volume, tank volume and the volume of liquid in the reactor. A typical flow diagram for a single stage MBBR process for BOD removal is shown in the figure below.



The equations for making those calculations are as follows:

1. BOD loading rate = Q*S_o*8.34*453.59

where: **Q** is the wastewater flow rate into the MBBR reactor in MGD

S_o is the BOD concentration in that influent flow in mg/L
8.34 is the conversion factor from mg/L to lb/MG
453.59 is the conversion factor from lb to g
The calculated BOD loading rate will be in g/day.

2. required carrier surf. area = BOD Loading Rate/SALR

where: BOD Loading Rate is in g/day

SALR is the design surface area loading rate in $g/m^2/day$ The calculated **required carrier surface area** will be in m².

3. required carrier volume = required carrier surf. area/carrier specific surf. area

where: **required carrier surface Area** is in m² **carrier specific surface Area** is in m²/m³ The calculated **required carrier volume** will be in m³.

4. required tank volume = required carrier volume/carrier fill %

where: required tank volume will be in the same units as required carrier volume.

5. liquid volume in tank = required tank volume – [required carrier volume(1 – carrier % void space)]

where: all three volumes will be in the same units.

Note that volumes calculated in m^3 can be converted to ft^3 by multiplying by 3.2808³ ft^3/m^3 .

Although hydraulic retention time (HRT) is not typically used as a primary design parameter for MBBR reactors, it can be calculated at the design wastewater flow rate, if the liquid volume in the tank is known. Also, if a design peak hour factor is specified, then the HRT at peak hourly flow can be calculated as well. The equations for calculating HRT are as follows:

1. Ave. HRT_{des ave} = liquid vol. in tank*7.48)/[Q*10⁶/(24*60)]

where: **liquid vol. in tank** is in ft³

Q is in MGD 7.48 is the conversion factor for ft³ to gal 10⁶ is the conversion factor for MG to gal 24*60 is the conversion factor for days to min Ave. HRT_{des ave} will be in min

2. Ave. HRT_{peak hr} = Ave. HRT_{des ave}/Peak Hour Factor

where: Ave. HRT_{peak hr} will also be in min

Table 1 below shows typical SALR design values for BOD removal in MBBR reactors. Reference #2 at the end of this course (Ødegaard, 1999) is the source for the values in the table.

Typical Design	Values for MBBR reactor	s at 15°C	
Purpose	Treatment Target % Removal	Design SALF g/m ² -d	
BOD Removal			
High Rate	75 - 80 (BOD7)	25 (BOD7)	
Normal Rate	85 - 90 (BOD7)	15 (BOD7)	
Low Rate	90 - 95 (BOD7)	7.5 (BOD7)	

Table 1. Typical Design	SALR Values f	for BOD Removal
Table II Typica Doolgi		

Example #1: A design wastewater flow of 1.5 MGD containing 175 mg/L BOD (in the primary effluent) is to be treated in an MBBR reactor.

- a) What is the BOD loading rate to the reactor in g/day?
- b) What would be a suitable design SALR to use for a target of 90-95% removal?
- c) If the MBBR carrier has a specific surface area of 600 m²/m³ and design carrier fill % of 40%, what would be the required volume of carrier and required MBBR tank volume?
- d) If the design carrier % void space is 60%, what would be the volume of liquid in the MBBR reactor?
- e) If the design peak hour factor is 4, calculate the average hydraulic retention time at design average wastewater flow and at design peak hourly wastewater flow.

Solution:

- a) The BOD loading rate will be (1.5 MGD)(175 mg/L)(8.34 lb/MG/mg/L) = 2189 lb/day = (2189 lb/day)*(453.59 g/lb) = <u>993,000 g BOD/day</u>
- b) From Table 1 above, a suitable design SALR value for BOD removal with a target BOD removal of 90–95% would be <u>7.5 g/m²/day</u>
- c) Required carrier surface area = $(993,000 \text{ g/day})/(7.5 \text{ g/m}^2/\text{day}) = 132,403 \text{ m}^2$. Required carrier volume = $132,403 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 220.7 m^3

For 40% carrier fill: Required tank volume = 220.7 m³/0.40 = 551.7 m^3 .

- d) The volume of liquid in the reactor can be calculated as: tank volume – [carrier volume(1 – void %)], Thus the volume of liquid is: 551.7 – [220.7(1 – 0.60)] = <u>463.4 m³</u> = 463.3*(3.2808³) = **16,365 ft³**
- e) The HRT at design ave ww flow can be calculated as: HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = $16,365*7.48/[1.5*10^6/(24*60)] = 118 \text{ min}$

HRT_{peak hr} = HRT_{des ave}/peak hour factor = 118/4 = <u>29 min</u>

Use of an Excel Spreadsheet: The type of calculations just discussed can be done conveniently using an Excel spreadsheet. The screenshot on the next page shows an example Excel spreadsheet set up to make the calculations as just described for **Example #1**.

The worksheet shown in the screenshot is set up for user input values to be entered in the blue cells with the values in the yellow cells calculated by the spreadsheet using the equations presented and discussed above. The values calculated by the spreadsheet for the following parameters are the same as those shown in the solution to **Example #1** above: i) the BOD loading in g/day, ii) the required tank volume, iii) the liquid volume in the tank, iv) the hydraulic retention time at design average flow, and v) the hydraulic retention time at peak hourly flow.

You may have noticed that there are a few additional calculations in the worksheet screenshot. If the user selects "rectangular" for the tank shape, the worksheet calculates the tank length and width for the calculated required tank volume. If the user selects cylindrical as the tank shape, the worksheet will calculate the tank diameter. These calculations simply use the formulas for the volume of a rectangular tank (V = L^*W^*H) or for the volume of a cylindrical tank (V = $\pi D^2H/4$), with user entered values for L:W ratio being used for rectangular tank calculations and the user entered value for the liquid depth in the tank, H, being used by both. The worksheet also calculates an estimated effluent BOD concentration. The effluent BOD calculation is discussed in the next section.

Estimation of Effluent Concentration: Use of an estimated surface area removal rate (SARR) allows calculation of the estimated effluent concentration of the parameter being removed. That is, for BOD removal, the estimated effluent BOD concentration can be calculated. For nitrification, the estimated effluent ammonia nitrogen concentration can be calculated and for denitrification, the estimated effluent nitrate nitrogen concentration can be calculated.

Instructions, Course		house	On mar data at	laulates	in well-	have
Instructions: Enter valu	es in blue	e boxes.	Spreadsheet ca	iculates values	in yellow	boxes
1. General Inputs						
		-	Data points	for SARR/SALR	vs SALR	
Design ww Flow Rate, Q =	1.5	MGD	SAL	LR (g/m ² /d):	7.5	15.0
Prim. Effl. BOD, So =	175	mg/L	SA	RR/SALR:	0.925	0.875
Peak Hour Factor =	4		(default values a values of % B	bove are based OD removal vs S		
Design Value of BOD Surface			Slope	, SARR/SALR vs	SALR:	-0.007
Area Loading Rate (SALR) =	7.5	g/m²/d		t, SARR/SALR vs		0.975
See information on typical of	lesign			Est. of SARR/SAL	1000	0.925
values for SALR below r	ght.		the second se	Removal Rate/Su value specified a		ding Rat
2. Calculation of Carrie	Volume	and Re	quired Tank Vo	olume & Dime	nsions	
Inputs			Liquid Dept	h in Tank =	8	ft
Carrier Spec. Surf. Area =	600	m ² /m ³		L:W ratio =	1.5	
(value from carrier mfr	/vendor)		(target L:W	only used if tan	k is rectang	jular)
Design Carrier Fill % =	40%	10 10	Click on green b	ox and then on		
(Carrier fill % is typically bet	ween 30%	and	arrow to Select	Tank Shape:	rectan	gular
70%. Lower values are mor			Carrier %	Void Space =	60%	
allowing future capacity exp	pansion or			ier mfr/vendor -	only neede	d to
reduction of SALR by adding			and the second se	e hydraulic dete	and a second sec	0.10
				1		
Calculations						
BOD Daily Loading =	2189	lb/day	Calculated	Tank Volume =	551.7	m ³
, , ,	993,022	g/day		=	19,482	ft ³
		37		=	145,723	gal
	132,403	m ²				3
Carrier Surf. Area needed =		m ³	Calculated	d Tank Width =	40.3	ft
	220.7			d Tank Length =	60.4	ft
	220.7		Calculated			
Calculated Carrier Volume =		ft ³		Complete States of the New York, States of the New Yor	e at	
	220.7 16,365		Nominal Hydrau	ulic Retention Tim	Contraction of the	min
Calculated Carrier Volume = Tank Liquid Volume =	16,365		Nominal Hydrau Design	ulic Retention Tim Average Flow =	118	min min
Calculated Carrier Volume =	16,365	ft ³	Nominal Hydrau Design	ulic Retention Tim	Contraction of the	1000000
Calculated Carrier Volume = Tank Liquid Volume = Estimate of BOD Surface Are	16,365 a		Nominal Hydrau Design Peal	ulic Retention Tim Average Flow =	118	1022308
Calculated Carrier Volume = Tank Liquid Volume = Estimate of BOD Surface Are Removal Rate, SARR =	16,365 a 6.94 918,545	ft ³ g/m ² /d g/day	Nominal Hydrau Design Peal Calculated	ulic Retention Tim Average Flow = k Hourly Flow = Effl BOD Conc.:	118 29 13	min mg/L
Estimate of BOD Surface Are Removal Rate, SARR =	16,365 a 6.94	ft ³ g/m²/d	Nominal Hydrau Design Peal Calculated If the calculated	ulic Retention Tim Average Flow = k Hourly Flow =	118 29 13 conc. is to	min mg/L

Screenshot of MBBR Process Design Calculations – Single Stage BOD Removal

Based on graphs and tables provided in several of the references at the end of this document, the SARR/SALR ratio for all of the different types of MBBR treatment being covered in this course ranges from about 0.8 to nearly 1.0 over the range of SALR values typically used. The SARR/SALR ratio is nearly one at very low SALR values and decreases as the SALR value increases.

The upper right portion of the screenshot on the previous page illustrates an approach for estimating a value for the SARR/SALR ratio for a specified design value of SALR. In the four blue cells at the upper right, two sets of values for SARR/SALR and SALR are entered. In this case they are based on the typical values of % BOD removal vs SALR in Table 1 above. In the yellow cells below those entries, the slope and intercept of a SARR/SALR vs SALR straight line are calculated using the Excel SLOPE and INTERCEPT functions. Then the SARR/SALR ratio is calculated for the specified design value of SALR.

Note that the ratio SARR/SALR is equal to the % BOD removal expressed as a fraction. This can be shown as follows:

BOD removal rate in g/day = (SARR in $g/m^2/d$)(Carrier Surf. Area in m^2)

BOD rate into plant in g/day = (SALR in $g/m^2/d$)(Carrier Surf. Area in m^2)

% **BOD removal** = (BOD removal rate/BOD rate into plant)*100%

= (100%)(SARR* Carrier Surf Area)/(SALR*Carrier Surf Area)

= (SARR/SALR)100%

Thus, the value of **0.925** for the **SARR/SALR** ratio at **SALR = 7.5 g/m²/d** was obtained from **Table 1** above as the midpoint of the 90-95% estimated % BOD removal for **SALR** = **7.5 g/m²/d**. Similarly, the value of **0.875** for the **SARR/SALR** ratio at **SALR = 15** g/m²/d was obtained from **Table 1** above as the midpoint of the 85-90% estimated % BOD removal for **SALR = 15 g/m²/d**.

At the bottom of the screenshot worksheet, the estimated value of the surface area removal rate (SARR) is calculated. It is used to calculate an estimated BOD removal rate in g BOD/day and lb BOD/day. Then an estimate of the effluent BOD concentration is calculated. The equations for these calculations are as follows:

- 1. estimated SARR = (calculated SARR/SALR)(design value of SALR)
- 2. estimated BOD removal rate = (estimated SARR)(carrier surface area)
- estimated effluent BOD conc.
 = [(BOD loading rate estimated BOD removal rate)/Q_o]/8.34

Example #2: Calculate the estimated effluent BOD concentration for the wastewater flow described in **Example #1** being treated in the MBBR reactor sized in **Example #1**.

Solution: The solution is included in the spreadsheet screenshot that was used for the solution to **Example #1** above. The pair of points for **SARR/SALR vs SALR** that were discussed above and are shown on the screenshot lead to the following values for the slope and intercept for the **SARR/SALR vs SALR** line: **Slope = -0.007, Intercept = 0.975**.

Thus the estimated SARR/SALR ratio for the given SALR value of 7.5 $g/m^2/d$ would be calculated as: SARR/SALR = - (0.007)(7.5) + 0.975 = 0.925

The SARR value can be calculated as:

SARR = $(SARR/SALR)(SALR) = (0.925)(7.5) = 6.94 \text{ g/m}^2/\text{d}$

Then, the estimated BOD removal rate can be calculated as:

est BOD removal rate = (est SARR)(carrier surface area)

 $= (6.94 \text{ g/m}^2/\text{d})(132,403 \text{ m}^2) = 918,545 \text{ g/d} = 918,545/453.59 \text{ lb/day}$

est BOD removal rate = 2025.1 lb/day

The estimated effluent BOD concentration can then be calculated from the equation:

est effluent BOD conc. = [(BOD loading rate - est BOD removal rate)/Q_o]/8.34

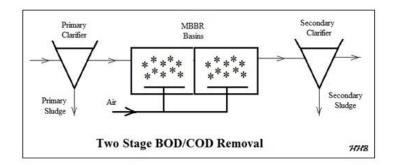
Substituting calculated and given values:

Est. effluent BOD conc. = [(2189 - 2026.1)/1.5]/8.34 = 13 mg/L

Note that this **13 mg/L** value for the **estimated effluent BOD** concentration is shown near the bottom of the spreadsheet screenshot.

6. Two-Stage BOD Removal MBBR Process Design Calculations

A two stage MBBR BOD removal process may be used instead of a single stage process. In this case, a high SALR "roughing" treatment will typically be used for the first stage and a lower SALR will typically be used for the second stage. This will result in less total tank volume needed for a two-stage process than for a single stage process. Also, a two-stage MBBR process can typically achieve a lower effluent BOD concentration than a single stage MBBR process. A typical flow diagram for a twostage MBBR process for BOD removal is shown in the figure below.



Two Stage MBBR Process for BOD/COD Removal

The process design calculations for a two stage MBBR process are essentially the same for each of the stages as for the single stage process, as described in the previous section. These calculations are illustrated in **Example #3**.

Example #3: A design wastewater flow of 1.5 MGD containing 175 mg/L BOD (in the primary effluent) is to be treated for BOD removal in a two-stage MBBR reactor. The SALR for the first stage is to be 25 g/m²/d and the design SALR for the second stage is to be 7.5 g/m²/d.

- a) For the first stage calculate each of the following:
 - i) The BOD loading
 - ii) The required carrier volume for a carrier with specific surface area of 600 m^2/m^3
 - iii) The required MBBR tank volume for a design carrier fill % of 40%
 - iv) The volume of liquid in the MBBR reactor for design carrier % void space of 60%.
 - v) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
 - vi) The estimated effluent BOD concentration from the first stage.
- b) Calculate the same parameters for the second stage.

Solution: The solution as calculated with an Excel spreadsheet is shown in the spreadsheet screenshot in the next two figures below.. The first screenshot below which is the top part of the spreadsheet, shows primarily the user input values. It also includes the calculation of the slope and intercept for the SARR/SALR vs SALR equation and the calculation of the estimated SARR for each stage. These calculations and the resulting SARR/SALR values are the same as those discussed above for the single-stage BOD removal MBBR process. The resulting values for SARR/SALR are **0.775** for the first stage with SALR = **25**, and **0.925** for the second stage with SALR = **7.5**.

The second screenshot figure below is the bottom part of the spreadsheet and shows the calculated values as follows.

- a) For the first stage:
- i) The BOD loading rate will be (1.5 MGD)(175 mg/L)(8.34 lb/MG/mg/L) = 2189 lb/day = (2189 lb/day)*(453.59 g/lb) = <u>993,022 g BOD/day</u>
- ii) Required carrier surface area = $(993,022 \text{ g/day})/(25 \text{ g/m}^2/\text{day}) = 39,721 \text{ m}^2$.

Required carrier volume = $39,721 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = <u>66.20 m³</u>

- iii) For 40% carrier fill: Required tank volume = $66.2 \text{ m}^3/0.40 = 165,5 \text{ m}^3$.
- iv) The volume of liquid in the reactor can be calculated as: tank volume – [carrier volume(1 – void %)], Thus the volume of liquid is: 165.5 – [66.20(1 – 0.60)] = $\underline{139.02 \text{ m}^3}$. = 139.02(3.2808³) = $\underline{4910 \text{ ft}^3}$
- v) The HRT at design ave ww flow can be calculated as: $HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = 4910*7.48/[1.5*10⁶/(24*60)] = 35 min$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 35/4 = <u>9 min</u>$

- vi) Calculation of the estimated effluent BOD concentration from the first stage as shown above for the single stage process gives a value of **39 mg/L**.
- b) For the second stage:
- i) The BOD loading rate will be (1.5 MGD)(39 mg/L)(8.34 lb/MG/mg/L) = 492.6 lb/day = (492.6 lb/day)*(453.59 g/lb) = <u>223,430 g BOD/day</u>
- ii) Required carrier surface area = $(223,430 \text{ g/day})/(7.5 \text{ g/m}^2/\text{day}) = 29,791 \text{ m}^2$.

Required carrier volume = 29,791 m²/600 m²/m³. = 49.65 m³

- iii) For 40% carrier fill: Required tank volume = 49.65 m³/0.40 = <u>124.1 m³</u>.
- iv) The volume of liquid in the reactor can be calculated as: tank volume – [carrier volume(1 – void %)], Thus the volume of liquid is: 124.1 $-[49.65(1 - 0.60)] = 104.3 \text{ m}^3 = 104.3(3.2808^3) = 3682 \text{ ft}^3$
- v) The HRT at design ave ww flow can be calculated as: $HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] =$ 3682*7.48/[1.5*10⁶/(24*60)] = 26 min

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 26/4 = 7 min$

Calculation of the estimated effluent BOD concentration from the second stage using the calculation procedure shown above for the single stage process gives a value of **3.0 mg/L**.

Instructions: Enter value	es in blu	e boxes.	Spreadsheet calculates values	in yellow	boxes	
				-		
I. Wastewater Parameter	Inputs					
1. Parameters for Both Fir	st and S	econd St	ane			
Design ww Flow Rate, Q =	1.5	MGD	Peak Hour Factor =	4		
2. Parameters for First Sta	ige:		Data points for SARR/SALR	s SALR		
			SALR (g/m ² /d):	7.5	25.0	
Prim. Effl. BOD, S _{o1} =	175	mg/L	SARR/SALR:	0.925	0.78	
			(default values above are based			
Design Value of BOD Surface			values of % BOD removal vs \$	SALR at the	e right)	
Area Loading Rate (SALR) =	25	g/m²/d	Slope, SARR/SALR vs	SALR:	-0.009	
See information on typical d	esign		Intercept, SARR/SALR vs	SALR:	0.989	
values for SALR below rig	ght.		Est. of SARR/SALR Rato = 0.775			
			(Surf. Area Removal Rate/Su	rf. Area Loa	ading Rate)	
3. Parameters for Second	Stage:					
				_		
Design Value of BOD Surface			Est. of SARR/SALR Rato =	0.925		
Area Loading Rate (SALR) =	7.5	g/m²/d	(Surf. Area Removal Rate/Su	rf. Area Loa	ding Rate)	
See information on typical d	esign					
values for SALR to the rig	ht.					
II. Carrier Parameter and	Tank S	Shape In	outs for both First and Seco	nd Stage	<u>s</u>	
Carrier Spec. Surf. Area =	600	m ² /m ³	Click on green box and then on			
(value from carrier mfr/	vendor)		arrow to Select Tank Shape:	recta	ngular	
Liquid Depth in Tank =	8	ft	Carrier % Void Space =	60%		
Tank L:W ratio =	1.5		(from carrier mfr/vendor -	only need	ed to	
(target L:W - only used if tan	k is recta	angular)	calculate hydraulic deter			

Screenshot of MBBR Process Design Calculations - Two Stage BOD Removal - Part 1

1. First Stage Calculation	s (BOD R	emoval)		(Carrier fill % is typi	ically be	etween 30%	and 70%.	Lower
				values are more co			-	
Design Carrier <mark>Fill</mark> % =	40%	(for first sta	age)	expansion or reduc	tion of	SALR by ad	Iding more	e carrie
BOD Daily Loading =	2189	lb/day						
	993,022	g/day		Calculated Tank Vol	ume =	165.5	m ³	
Carrier Surf. Area needed =	39,721	m ²			=	5844.7	ft ³	
Calculated Carrier Volume =	66.20	m ³			=	43718	gal	
Tank Liquid Volume =	4910	ft ³		Calculated Tank Wi	dth =	22.1	ft	
	139.0231	4909.374		Calculated Tank Ler	ngth =	33.1	ft	
Estimate of BOD Surface Are	a		No	ominal Hydraulic Reter	ntion Tim	e at		
Removal Rate, SARR =	19.38	g/m²/d		Design Average F	=low =	35	min	
Est. of BOD Removal Rate:	769,592	g/day		Peak Hourly F	low =	9	min	
	1696.7	lb/day		Calculated Effl BOD	Conc.:	39	mg/L	
				(from First Stage)				
2. Second Stage Calculat	ions (BOI	D Remova	<u>d)</u>					
Design Carrier Fill % =	40%	(for second	d stag	ge)				
BOD Daily Loading =	492.6	lb/day		Calculated Tank Vol	ume =	124.1	m ³	
	223,430	g/day			=	4383.5	ft ³	
Carrier Surf. Area needed =	29,791	m ²				32789	gal	
Calculated Carrier Volume =	49.65	m ³		Calculated Tank Wi	dth =	19.1	ft	
	3682	ft ³		Calculated Tank Ler	ngth =	28.7	ft	
Tank Liquid Volume =			No	ominal Hydraulic Retention Tim		e at		
Tank Liquid Volume = 104.2673003	3682.031			Design Average Flow =				
	1.1000000000000000000000000000000000000			1		26	min	
104.2673003	1.1000000000000000000000000000000000000	g/m²/d		1	low =	26 7	min min	
104.2673003 Estimate of BOD Surface Are	a	g/m²/d g/day		Design Average f	=low = Tow =		13	
104.2673003 Estimate of BOD Surface Are Removal Rate, SARR =	6.94	g/day		Design Average F Peak Hourly F	Flow = Flow = Conc.:	7	min	
104.2673003 Estimate of BOD Surface Are Removal Rate, SARR = Est. of BOD Removal Rate:	ea 6.94 206,673			Design Average f Peak Hourly F Calculated Effl BOD	Flow = Flow = Conc.:	7 3.0	min mg/L	r
104.2673003 Estimate of BOD Surface Are Removal Rate, SARR =	ea 6.94 206,673	g/day Ib/day	lf th	Design Average F Peak Hourly F Calculated Effl BOD (from Second Stage	Flow = Flow = Conc.: DD conc	7 3.0 . is too higl	min mg/L n for eithe	r?

Screenshot of MBBR Process Design Calculations - Two Stage BOD Removal - Part 2

Example #4: Compare the MBBR tank volume, carrier surface area, and estimated effluent BOD concentration for the single stage BOD removal process in **Example #1** with the two-stage BOD removal process treating the same wastewater flow and influent BOD concentration as calculated in **Example #3**.

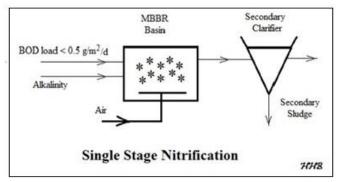
Solution: The results are summarized below:

	Single Stage Process	Two-Stage Process
MBBR Volume:	19,482 ft ³	10,228 ft ³
Carrier Surf. Area:	132,403 m ²	69,512 m ²
Est. Effl. BOD:	13 mg/L	3 mg/L

Note that the two-stage process requires only about half of the tank volume and half of the carrier quantity in comparison with the single stage process, while achieving a significantly lower estimated effluent BOD.

7. Single Stage Nitrification MBBR Process Design Calculations

An MBBR single stage nitrification process would typically be used as a tertiary treatment process following some type of secondary treatment that reduced the BOD to a suitable level. A typical flow diagram for a single stage MBBR process for nitrification is shown in the figure below. As shown on the diagram, the BOD level should be low enough so that the BOD load to the nitrification process is less than 0.5 g/m²/day. Note that alkalinity is used in the nitrification process and thus alkalinity addition is typically required.



Single Stage MBBR Process for Nitrification

The process design calculations for this single stage MBBR process are similar to those used for the BOD removal processes, but the design SALR value for nitrification can be calculated rather than being selected from a table of typical values, as was done for BOD removal. The design SALR can be calculated using a kinetic model for the surface area removal rate (SARR) as a function of the dissolved oxygen concentration in the MBBR reactor and the bulk liquid ammonia nitrogen concentration, which is equal

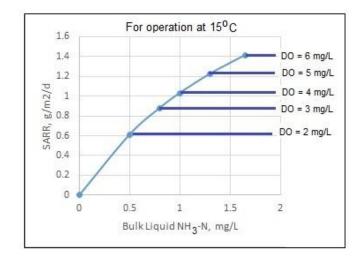
to the effluent ammonia nitrogen concentration assuming completely mixed conditions in the MBBR tank.

The kinetic model to be discussed here is from Metcalf and Eddy (2014), Figure 9-25 [attributed to Odegaard (2006)] and Equation 9-48. This figure and equation will now be shown and discussed briefly. The figure below was prepared based on Metcalf and Eddy's Figure 9-25 and their Equation 9-48 (shown below). Note that the figure and equation are for operation at 15°C. Correction of the SARR and SALR for some other operational temperature can be done with the equation: SARR_T = SARR₁₅ $\theta^{(T-15)}$ where T is the design operational temperature in °C. From Salvetti, et.al (2006): θ = **1.058** for D.O. limited conditions and θ = **1.098** if ammonia nitrogen concentration is the limiting factor.

Equation 9-48 from Metcalf and Eddy (2014) is:

```
SARR = [N_e/(2.2 + N_e)]^*3.3, N_e = effluent ammonia N conc.
```

This is the equation for the curved portion at the left in the graph below, which is for ammonia N concentration limiting conditions.



Adapted from Metcalf & Eddy (2014), Fig 9-25

The horizontal lines in the figure above show the nitrification SARR under D.O. limiting conditions for each of the D.O. levels shown. The SARR will be D.O. limited when the NH₃-N concentration is above the value at the left end of the horizontal line for each D.O. level. When the NH₃-N concentration is below that value, then the SARR is ammonia concentration limited and the SARR is a function of the effluent NH₃-N concentration (N_e) per the equation: SARR = [N_e/(2.2 + N_e)]*3.3.

As shown in **Section 5** for BOD removal, **SALR/SARR = % BOD removal**. Similarly, for nitrification, **SALR/SARR = % NH₃-N removal**. After the **SARR** has been determined, the **SALR** can be calculated as: **SALR = SARR/% removal**.

The maximum **SARR** for each of the D.O. levels shown in the figure above are shown in **Table 2** below, along with the ammonia nitrogen concentration above which the **SARR** will be at that maximum value.

D.O.	SARR _{max}	min NH ₃ -N _e @ SARR _{max}
mg/L	g/m ² /d	mg/L
2	0.61	0.5
3	0.88	0.8
4	1.03	1
5	1.23	1.3
6	1.41	1.65

Table 2. Values of $SARR_{max}$ and $NH_3\text{-}N_e @ SARR_{max}$

Example #5: A design flow of 0.2 MGD has the following characteristics: 15 mg/L BOD, 25 mg/L NH₃-N, and alkalinity of 140 mg/L as CaCO₃. This flow is to be treated in a single stage nitrification MBBR reactor. The target effluent NH₃-N is 3.3 mg/L. The dissolved oxygen is to be maintained at 3.0 mg/L in the MBBR reactor. The design minimum wastewater temperature is to be 45° F.

- a) Determine an appropriate NH₃-N surface area loading rate (SALR) for this nitrification process, in g NH₃-N/m²/d.
- b) What is the NH₃-N loading rate to the reactor in g/day?
- c) If the MBBR carrier has a specific surface area of 600 m²/m³ and design carrier fill % of 40%, what would be the required volume of carrier and required MBBR tank volume?
- d) Calculate the BOD surface area loading rate (SALR) to ensure that it is less than 0.5 g BOD/m²/day.
- e) If the design carrier % void space is 60%, what would be the volume of liquid in the MBBR reactor.
- f) If the design peak hour factor is 4, calculate the average hydraulic retention time at design average wastewater flow and at design peak hourly wastewater flow.
- g) Calculate the alkalinity requirement in lb/day as CaCO₃ and in lb/day NaHCO₃ for a target effluent alkalinity of 80 mg/L.

Solution:

The solution is shown in the spreadsheet screenshots below. A summary of the calculations is as follows:

a) The D.O. limited SARR can be obtained as SARR_{max} from Table 2 for the specified D.O level, and the minimum ammonia nitrogen concentration for that SARR value can be obtained from the same table. The values from Table 2, for a D.O. of 3.0 are: SARR_{max} = 0.88 g/m²/d and minimum NH₃-N_e for that value of SARR_{max} = 0.80 mg/L. (in the worksheet shown in the first screenshot below, these two values are obtained using Excel's VLOOKUP function from a table like Table 2, above that is on the worksheet.

The **SARR** for the design D.O. and ammonia nitrogen removal at 15° C will then be equal to **SARR**_{max} if the target effluent ammonia nitrogen concentration is greater than the 0.80 mg/L value determined above. If the target effluent ammonia nitrogen concentration is less than 0.80 mg/L, then the SARR needs to be calculated using Metcalf & Eddy's equation 9-48. In this case, the target effluent NH₃-N of 3.3 mg/L is greater than 0.8 mg/L, so the SARR at 15°C is 0.88 g/m²/d.

The design value for the SARR at the design minimum wastewater temperature can then be calculated as: $SARR_T = SARR_{15}\theta^{(T-15)}$, where the WW temperature must be in °C. Since this case has D.O. limited conditions, $\theta = 1.058$. Carrying out this calculation gives: **design value of SALR = 0.65** g/m²/d.

- b) The ammonia nitrogen loading rate will be (0.2 MGD)(25 mg/L)(8.34 lb/MG/mg/L) = 41.7 lb/day = (41.7 lb/day)*(453.59 g/lb) = <u>18,915 g NH₃-N/day</u>
- c) Required carrier surface area = $(18,915 \text{ g/day})/(0.65 \text{ g/m}^2/\text{day}) = 28,925 \text{ m}^2$.

Required carrier volume = 28,925 m²/600 m²/m³. = 48.209 m³ = (48.209 m³)(3.2808³ ft³/m³) = <u>1702 ft³</u>.

For 40% carrier fill: Required tank volume = $1702 \text{ ft}^3/0.40 = 4256 \text{ ft}^3$.

- d) The BOD SALR will be $(0.2 \text{ MGD})(15 \text{ mg/L})(8.34 \text{ lb/MG/mg/L})(453.59)/(28925 \text{ m}^2) = 0.39 \text{ g/m}^2/\text{day}$ (Note that this is less than 0.5 g/m²/day as required.)
- e) The volume of liquid in the reactor can be calculated as: tank volume – [carrier volume(1 – void %)], Thus the volume of liquid is: 4256 – [1702(1 – 0.60)] = <u>3575 ft³</u>.

f) The HRT at design ave ww flow can be calculated as: $HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = 3575*7.48/[0.2*10⁶/(24*60)] = <u>193 min</u>$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 193/4 = 48 min$

g) Calculation of the alkalinity requirement is shown in the second screenshot below. The alkalinity needed for nitrification is 7.14 g CaCO₃/g NH₃-N removed. Thus, the alkalinity to be added in mg/L can be calculated as:

7.14(mg/L NH₃-N removed) + target effl. Alkalinity – influent alkalinity

For the given input values, this becomes:

 $7.14(25 - 3.3) + 80 - 140 = 94.9 \text{ mg/L} \text{ as } CaCO_3$

The rate of alkalinity addition needed can then be calculated as:

 $(0.2 \text{ MGD})(94.9 \text{ mg/L})*8.34 = 158.4 \text{ lb/day as CaCO}_3.$

Multiplying this by the ratio of the equivalent weight of $NaHCO_3$ (84) to the equivalent weight of $CaCO_3$ (50) gives the daily $NaHCO_3$ requirement as

266.0 lb/day NaHCO3.

			_			
Instructions: Enter valu	es in blu	e boxes.	Sprea	dsheet calculates values in	yellow bo	xes
1. General Inputs				Peak Hour Factor =	4	1
			Target	Effl NH ₄ -N Conc, \mathbf{NH}_4 -N _e =	3.3	mg/L
Design ww Flow Rate, Q =	0.2	MGD		Min Design Temp., T =	45	°F
Influent NH ₄ -N Conc. =	25	mg/L				0-88 51
Influent BOD, So =	15	mg/L		Click on cell H10 and on a	rrow to sel	ect D.O C
Prim. Effl. Alkalinity =	140	mg/L as	CaCO ₃	D.O Conc. in Reactor =	3.0	mg/L
2. Preliminary Calculati	ons - De	sign SA	L <mark>R va</mark> l	ue		
% NH ₄ -N removal =	87%			NH4-Ne @ SARRmax =	0.80	mg/L
Maximum SARR =	0.88	g/m²/d	SARR	@ NH4-Ne, 15°C, SARR15 =	0.88	g/m²/c
			SARR	@ NH ₄ -Ne, T °C, SARR _T =	0.57	g/m²/c
SARR Temp. Coeff, 0 =	1.058					
021				Design Value for SALR =	0.65	g/m²/d
Inputs		20				
Carrier Spec. Surf. Area =	600	m ² /m ³		Liquid Depth in Tank =	8	ft
Inputs Carrier Spec. Surf. Area = (value from carrier mfr		m²/m³		Tank L:W ratio =	1.5	
Carrier Spec. Surf. Area =		m²/m³	(t	the state processing of the state of the sta	1.5	
Carrier Spec. Surf. Area = (value from carrier mfr		m²/m³		Tank L:W ratio =	1.5	
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % =	/vendor) 40%		Click	Tank L:W ratio = arget L:W - only used if tank is	1.5	ar)
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet	/vendor) 40% tween 30%	and	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on	1.5 s rectangula	ar)
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet 70%. Lower values are mor	/vendor) 40% tween 30% e conserva	and ative,	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape:	1.5 s rectangula rectar 60%	ar) ngular
Carrier Spec. Surf. Area =	/vendor) 40% tween 30% e conserva pansion of	and ative,	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape: Carrier % Void Space =	1.5 s rectangula rectar 60% ly needed t	ar) ngular
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet 70%. Lower values are mor allowing future capacity exp reduction of SALR by adding	/vendor) 40% tween 30% e conserva pansion of	and ative,	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape: Carrier % Void Space = (from carrier mfr/vendor - on calculate hydraulic detention	1.5 s rectangula rectar 60% ly needed t on time)	ngular
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet 70%. Lower values are mor allowing future capacity exp reduction of SALR by adding Calculations	/vendor) 40% tween 30% e conserva pansion of g more ca	and ative, rrier.)	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape: Carrier % Void Space = (from carrier mfr/vendor - on calculate hydraulic detention Calculate Tank Volume =	1.5 s rectangula rectar 60% ly needed t on time) 120.5	ngular to m ³
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet 70%. Lower values are mor allowing future capacity exp reduction of SALR by adding	/vendor) 40% tween 30% e conserva pansion of g more ca 41.7	and ative, rrier.) Ib/day	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape: Carrier % Void Space = (from carrier mfr/vendor - on calculate hydraulic detentio Calculated Tank Volume = =	1.5 s rectangula rectar 60% ly needed t on time) 120.5 4256.2	mgular to m ³ ft ³
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet 70%. Lower values are mor allowing future capacity exp reduction of SALR by adding Calculations NH ₃ -N Daily Loading =	/vendor) 40% tween 30% e conserva pansion of g more ca 41.7 18915	and ative, rrier.) Ib/day g/day	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape: Carrier % Void Space = (from carrier mfr/vendor - on calculate hydraulic detentio Calculated Tank Volume = = =	1.5 s rectangula rectar 60% ly needed t on time) 120.5 4256.2 31837	ar) agular co m ³ ft ³ gal
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet 70%. Lower values are mor allowing future capacity exp reduction of SALR by adding Calculations NH ₃ -N Daily Loading =	40% 40% tween 30% e conserve pansion of g more ca 41.7 18915 28925	and ative, rrier.) Ib/day g/day m ²	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape: Carrier % Void Space = (from carrier mfr/vendor - on calculate hydraulic detention Calculated Tank Volume = = Calculated Tank Width =	1.5 s rectangula rectar 60% ly needed to on time) 120.5 4256.2 31837 18.8	ar) gular io m ³ ft ³ gal ft
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet 70%. Lower values are mor allowing future capacity exp reduction of SALR by adding Calculations NH ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume =	40% 40% ween 30% e conserva pansion of g more ca 41.7 18915 28925 48.209	and ative, rrier.) Ib/day g/day m ² m ³	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape: Carrier % Void Space = (from carrier mfr/vendor - on calculate hydraulic detention Calculated Tank Volume =	1.5 s rectangula 60% ly needed t on time) 120.5 4256.2 31837 18.8 28.2	ar) agular co m ³ ft ³ gal
Carrier Spec. Surf. Area = (value from carrier mfr Design Carrier Fill % = (Carrier fill % is typically bet 70%. Lower values are mor allowing future capacity exp reduction of SALR by adding Calculations	40% 40% tween 30% e conserve pansion of g more ca 41.7 18915 28925	and ative, rrier.) Ib/day g/day m ²	Click	Tank L:W ratio = arget L:W - only used if tank is on green box and then on w to Select Tank Shape: Carrier % Void Space = (from carrier mfr/vendor - on calculate hydraulic detention Calculated Tank Volume = = Calculated Tank Width =	1.5 s rectangula 60% ly needed t on time) 120.5 4256.2 31837 18.8 28.2	m ³ tt ³ gal ft

Screenshot of MBBR Process Design Calculations - Single Stage Nitrification - Part 1

5. Alkalinity Requi	rement				-		
Input:	Target Effl	uent Alkalinity	· =	80	mg/L		
Constants needed	I for Calculatio	ons:					
Equiv Wt. of CaCC	o ₃ = 50	g/equiv.		Equiv Wt.	of NaHCO ₃ =	84	g/equiv.
Alkalinity used fo	r Nitrification =	7.14	g Ca	CO ₃ /g NH ₃ -N			
Calculations							
Alkalinity	to be added =	94.9	mg/L	as CaCO3			
Daily Alkalinity	/ Requirement =	158.4	lb/da	y as CaCO₃			
For sodium bicar	bonate use to a	dd alkalinity					
Daily NaHCO ₃	Requirement =	266.0	lb/da	y NaHCO3		1	

Screenshot of MBBR Process Design Calculations – Single Stage Nitrification – Part 2

8. Two-Stage BOD Removal and Nitrification MBBR Process Design Calculations

A two stage MBBR process may also be used to achieve both BOD removal and nitrification. Nitrification with an MBBR process requires a rather low BOD concentration in order to favor the nitrifying bacteria in the biomass attached to the carrier. Thus, the first stage for this process is used for BOD removal and the second stage is used for nitrification. A typical flow diagram for a two stage MBBR process for BOD removal and nitrification is shown in the figure below. As in the single stage nitrification process alkalinity is used for nitrification, so alkalinity addition is typically required.

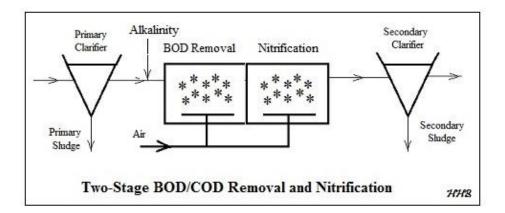


Table 3 shows typical design values for the SALR (surface area loading rate) for the BOD removal stage and for the nitrification stage. The source for the values in this table is the Odegaard reference #2 below. The design SALR value for nitrification, however, will be calculated based on the design D.O concentration and the target effluent NH₃-N concentration, as it was for the single-stage nitrification process.

Typical Design Values for	MBBR reactors at 15°C				
Purpose	Treatment Target % Removal	Design SALR g/m ² -d			
Nitrification BOD removal stage	90 - 95 (BOD ₇)	6.0 (BOD7)			
Effl. NH ₃ -N > 3 mg/L	90 (NH ₃ -N)	1.00 (NH ₃ -N)			
Effl. NH ₃ -N < 3 mg/L	90 (NH ₃ -N)	0.45 (NH ₃ -N)			

Table 3. Typical Design SALR Values for Nitrification
--

The process design calculations for this two stage MBBR process are essentially the same as those described above for the previous examples. **Table 3** will be used to obtain values for SARR/SALR vs SALR for the BOD removal stage as in the previous examples. The design SALR for the nitrification stage will be calculated as it was for the single stage nitrification process.

The process design calculations for this two-stage process are illustrated in **Example #6**.

Example #6: A design wastewater flow of 1.5 MGD has the following primary effluent characteristics: 175 mg/L BOD, 35 mg/L TKN, and alkalinity of 140 mg/L as CaCO₃. The design minimum wastewater temperature is 45° F. This flow is to be treated for BOD removal and nitrification in a two stage MBBR process. The design SALR for the first stage is to be 6 g BOD/m²/d, the target effluent NH3-N from the second stage is to be 3.3 mg/L and the design DO level in the nitrification stage is to be 3.0 mg/L.

a) For the first (BOD removal) stage calculate each of the following:

- i) The BOD loading
- ii) The required carrier volume for a carrier with specific surface area of $600 \text{ m}^2/\text{m}^3$
- iii) The required MBBR tank volume for a design carrier fill % of 40%

- iv) The volume of liquid in the MBBR reactor for design carrier % void space of 60%.
- v) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
- vi) The estimated effluent BOD concentration from the first stage.
- b) For the second (Nitrification) stage calculate each of the following:
 - i) An appropriate NH_3 -N surface area loading rate (SALR) to be used for this nitrification process, in g NH_3 -N/m²/d.
 - ii) The nitrate loading
 - iii) The required carrier volume for a carrier with specific surface area of 600 m^2/m^3
 - iv) The required MBBR tank volume for a design carrier fill % of 40%
 - v) The volume of liquid in the MBBR reactor for design carrier void space of 60%.
 - vi) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
 - vii) The alkalinity requirement in lb/day as CaCO₃ and in lb/day NaHCO₃.

Solution: The solution, as calculated with an Excel spreadsheet, is shown in the three spreadsheet screenshots on the next several pages. The first screenshot, which is the top part of the spreadsheet, shows primarily the user input values. It includes the calculation of the slope and intercept for the SARR/SALR vs SALR equation and the calculation of the estimated SARR for the BOD removal stage. It also includes calculation of the design SALR value for the nitrification stage. These calculations are carried out as discussed above for the single-stage BOD removal and nitrification MBBR processes. The resulting values for SARR/SALR are **0.935** for the first stage with SALR = **6** g BOD/m²/d, and **0.63** g/m2/d for the second stage (nitrification) SALR.

The second screenshot is the bottom part of the spreadsheet and shows the calculated values as follows.

- a) For the first stage:
- i) The BOD loading rate will be (1.5 MGD)(175 mg/L)(8.34 lb/MG/mg/L) = 2,189 lb/day = (2189 lb/day)*(453.59 g/lb) = <u>993,022 g BOD/day</u>
- ii) Required carrier surface area = $(993022 \text{ g/day})/(6 \text{ g/m}^2/\text{day}) = 165,504 \text{ m}^2$.

Required carrier volume = $165,504 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 275.8 m^3

iii) For 40% carrier fill: Required tank volume = $275.8 \text{ m}^3/0.40 = \underline{689.6 \text{ m}^3}$.

iv) The volume of liquid in the reactor can be calculated as:

tank volume – [carrier volume(1 – void %)], Thus the volume of liquid is: 689.6 – $[275.8(1 - 0.60)] = 579.3 \text{ m}^3 = 579.3(3.2808^3) = 20,456 \text{ ft}^3$.

v) The HRT at design ave ww flow can be calculated as: $HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = 20,456*7.48/[1.5*10⁶/(24*60)] = 147 min$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 147/4 = 37 min$

Instructions: Enter valu	es in blue	boxes.	Spread	dsheet calcula	ates values ir	n yellow b	oxes
I. Wastewater Paramete	r Inputs					-	-
1. Parameters for Both Fi	rst and Se	cond St	age				
Design ww Flow Rate, Q =	1.5	MGD		Peak Hour	Factor =	4	
		Pi	im. Effl.	Alkalinity =	140	mg/L as (CaCO₃
2. Parameters for First St	ade.		-	Data points for	SARR/SALR V	SALR	
2. I didileters for i list st	uge.		1	Data points for SARR/SALR v SALR (g/m ² /d):		7.5	15.0
Prim. Effl. BOD, So1 =	175	mg/L		SARR/SALR:		0.925	0.875
				It values above			
Design Value of BOD Surface			values of % BOD removal vs SALR in Worksheet				ksheet 3)
Area Loading Rate (SALR) =	6	g/m²/d		Slope, S/	ARR/SALR vs \$	SALR:	-0.007
See information on typical of	lesign			Intercept, SARR/SALR vs S		SALR:	0.975
values for SALR at the right.				Est. of SARR/SALR Ratio =			
				(Surf. Area Rei	moval Rate/Sur	f. Area Loa	ding Rate)
3. Parameters for Second	Stage:						
Influent NH ₄ -N Conc. =	35	mg/L	Target	Effl NH ₄ -N Conc	, NH ₄ -N _e =	3.3	mg/L
Min Design Temp., T =	45	°F	3000-67-515	k on cell H26 a		o select D.	O Conc.
	· · · · · · · · · · · · · · · · · · ·			D O Conc	. in Reactor =	3.0	mg/L
4. Preliminary Calculati	ons - Des	ign Nitr	ificatio				
% NH ₄ -N removal =	91%			NH4-Ne @ SA	RR _{max} =	0.80	mg/L
Maximum SARR =	0.88	g/m²/d	SARR	@ NH ₄ -Ne, 15°0	C, SARR 15 =	0.88	g/m²/d
			SARR	@ NH ₄ -Ne, T °C	, sarr _t =	0.57	g/m²/d
	1.058	D	acian V	alua for nitrificati	ion SALR =	0.63	g/m ² /d

Screenshot of MBBR Process Design Calculations for Two Stage BOD Removal and Nitrification – Part 1 vi) Calculation of the estimated effluent BOD concentration from the first stage as shown above for the single stage BOD removal process gives a value of **11 mg/L**.

- b) For the second stage:
- i) The design value for the nitrification **SALR** is **0.63** g/m²/d, calculated in the same way as it was for the single stage nitrification process.

1. First Stage Calculation	s - BOD R	emoval		(Carrier fill % is				
				values are mor		and the second se		
Design Carrier Fill % =	40%	(for first sta	ige)	expansion or r	eduction of S	SALR by add	ding more	carrie
BOD Daily Loading =	2189.3	lb/day						
	993022	g/day		Calculated Tank	k Volume =	689.6	m ³	
Carrier Surf. Area needed =	165503.7	m ²			1. .	24352.9	ft ³	
Calculated Carrier Volume =	275.839	m ³			=	182160	gal	
Tank Liquid Volume =	20456.5	ft ³		Calculated Tan	k Width =	45.0	ft	
				Calculated Tan	k Length =	67.6	ft	
Estimate of BOD Surface Are	a		Nom	inal Hydraulic R	etention Time	at		
Removal Rate, SARR =	5.61	g/m²/d		Design Aver	age Flow =	147	min	
Est. of BOD Removal Rate:	928475	g/day		Peak Hourly Flow =		37	min	
	2046.9	lb/day		Calculated Effl BOD Conc.:		11	mg/L	
				(from First St	age)			
If the calculated Effl. BOD of	conc. is too	high, the d	lesign	value of the SA	ALR should b	e reduced i	n cell C18	
		141 +1						
2. Second Stage Calculat	tions - Niti	ification						
2. Second Stage Calculat	tions - Niti	itication		Calculated Tanl	k Volume =	1320.5	m ³	
	40%	(for second	l stage		k Volume = =	1320.5 46632.1	m ³ ft ³	
			l stage					
Design Carrier Fill % =	40%	(for second	l stage		=	46632.1	ft ³	
Design Carrier Fill % = NH ₃ -N Daily Loading =	40% 437.9	(for second Ib/day	l stage)	= = k Width =	46632.1 348808	ft ³ gal	
Design Carrier Fill % = NH ₃ -N Daily Loading = Carrier Surf. Area needed =	40% 437.9 198604	(for second Ib/day g/day) Calculated Tan	= = k Width = k Length =	46632.1 348808 62.3 93.5	ft ³ gal ft	
Carrier Surf. Area needed = Calculated Carrier Volume =	40% 437.9 198604 316914.0 528.190	(for second lb/day g/day m ²) Calculated Tan Calculated Tan inal Hydraulic R	= = k Width = k Length = etention Time	46632.1 348808 62.3 93.5 at	ft ³ gal ft	
Design Carrier Fill % = NH ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume =	40% 437.9 198604 316914.0	(for second Ib/day g/day m ² m ³) Calculated Tan Calculated Tan iinal Hydraulic R Design Aver	= k Width = k Length = etention Time age Flow =	46632.1 348808 62.3 93.5	ft ³ gal ft ft	
Design Carrier Fill % = NH ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume =	40% 437.9 198604 316914.0 528.190 42736	(for second Ib/day g/day m ² m ³) Calculated Tan Calculated Tan inal Hydraulic R Design Aver	= = k Width = k Length = etention Time	46632.1 348808 62.3 93.5 at 307	ft ³ gal ft ft min	
Design Carrier Fill % = NH ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = 1st stage tank volume - - 2nd stage tank volume =	40% 437.9 198604 316914.0 528.190 42736 -630.9	(for second lb/day g/day m ² m ³ ft ³	Nor) Calculated Tan Calculated Tan inal Hydraulic R Design Aver Peak Hou	= k Width = k Length = etention Time age Flow =	46632.1 348808 62.3 93.5 at 307	ft ³ gal ft ft min	
Design Carrier Fill % = NH ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = 1st stage tank volume -	40% 437.9 198604 316914.0 528.190 42736 -630.9 volume th	(for second Ib/day g/day m ² m ³ ft ³ e same as t	Nom) Calculated Tan Calculated Tan inal Hydraulic R Design Aver Peak Hou Check on I	= k Width = k Length = etention Time age Flow = urly Flow =	46632.1 348808 62.3 93.5 at 307 77 0.2	ft ³ gal ft ft min min g/m ² /d	

Screenshot of MBBR Process Design Calculations for Two Stage BOD Removal and Nitrification – Part 2

- ii) The NH₃-N loading rate will be (1.5 MGD)(35 mg/L)(8.34 lb/MG/mg/L) = 437.9 lb/day = (437.9 lb/day)*(453.59 g/lb) = <u>198,604 g NH₃-N/day</u>
- iii) Required carrier surface area = $(198,604 \text{ g/day})/(0.63 \text{ g/m}^2/\text{day})$ = 316,914 m².

Required carrier volume = $316914 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 528.19 m^3 = $(528.19 \text{ m}^3)(3.2808^3 \text{ ft}^3/\text{m}^3)$ = $18,652 \text{ ft}^3$.

- iv) For 40% carrier fill: Required tank volume = $18652 \text{ ft}^3/0.40$ = $\underline{46,632 \text{ ft}^3}$.
- v) The volume of liquid in the reactor can be calculated as: tank volume – [carrier volume(1 – void %)], Thus the volume of liquid is: 46632 – [18652(1 – 0.60)] = <u>42,736 ft³</u>.
- vi) The HRT at design ave ww flow can be calculated as: $HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = 42736*7.48/[1.5*10⁶/(24*60)] = <u>307 min</u>$

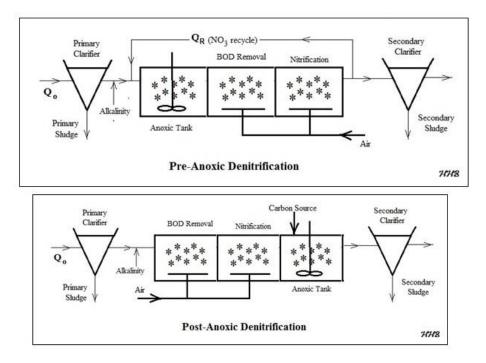
 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 307/4 = 77 min$

V. Calculation of	Alkalinity Red	quirements	5				
Input:	Target Efflue	ent Alkalinity =		80	mg/L		
Constants needed	d for Calculatio	ns:					
Equiv Wt. of CaCC	D ₃ = 50	g/equiv.		Equiv Wt.	of NaHCO ₃ =	84	g/equiv.
Alkalinity used fo	or Nitrification =	7.14	g CaCC	₃/g NH₃-N			
Calculations							
Alkalinity	to be added =	166.3	mg/L a	s CaCO ₃			
Daily Alkalinit	y Requirement =	2080.9	lb/day	as CaCO ₃			
For sodium bicar	rbonate use to a	dd alkalinity:					
Daily NaHCO ₃	Requirement =	3495.9	lb/day	NaHCO ₃			

Screenshot of MBBR Process Design Calculations for Two Stage BOD Removal and Nitrification – Part 3 vii) Calculation of the alkalinity requirement is shown in part 3 of spreadsheet screenshot, shown above. Using the equivalent weight of CaCO₃ as 50, the equivalent weight of NaHCO₃ as 84, the alkalinity use for nitrification as 7.14 g CaCO₃/g NH₃-N and the target effluent alkalinity as 80 mg/L as CaCO₃, give the calculated alkalinity requirement as 166.3 mg/L as CaCO₃. The rate of alkalinity addition needed can then be calculated as: (1.5 MGD)(166.3 mg/L)*8.34 = 2080.9 Ib/day as CaCO₃. Multiplying this by the ratio of the equivalent weight of NaHCO₃ to the equivalent weight of CaCO₃ gives the daily NaHCO₃ requirement as 3495.9 Ib/day NaHCO₃.

9. Denitrification Background Information

In order to carry out denitrification of a wastewater flow (removal of the nitrogen from the wastewater), it is necessary to first nitrify the wastewater, that is, convert the ammonia nitrogen typically present in the influent wastewater to nitrate. Nitrification will only take place at a reasonable rate in an MBBR reactor if the BOD level is quite low. Thus, an MBBR denitrification process will need a reactor for BOD removal, one for nitrification, and one for denitrification. The nitrification reactor will always follow the BOD removal reactor, because of the need for a low BOD level in the nitrification reactor (called pre-anoxic denitrification) or after the nitrification reactor (called post-anoxic denitrification). Flow diagrams for these two denitrification options are shown in the figure below.



MBBR Denitrification Process Alternatives

A bit more information about the denitrification reactions will be useful for further discussion of these two denitrification options. The denitrification reactions, which convert nitrate ion to nitrogen gas, and hence remove it from the wastewater flow, will take place only in the absence of oxygen, that is, in an anoxic reactor. Also, the denitrification reactions require a carbon source. With these factors in mind the functioning of the pre-anoxic denitrification process and of the post-anoxic denitrification process are described in the following two paragraphs.

In a **pre-anoxic denitrification** process, the BOD in the primary effluent wastewater is used as the carbon source for denitrification. In this process, however, the primary effluent entering the pre-anoxic reactor still has ammonia nitrogen present, rather than the nitrate nitrogen needed for denitrification. A recycle flow of effluent from the nitrification reactor is used to send nitrate nitrogen to the anoxic denitrification reactor as shown in the flow diagram above.

In a **post-anoxic denitrification** process, the influent to the denitrification reactor comes from the nitrification reactor, so the wastewater influent ammonia nitrogen has been converted to nitrate as required for denitrification. The BOD has also been removed prior to the post anoxic denitrification reactor, however, so an external carbon source is required for the denitrification reactions. Methanol is a commonly used carbon source for post-anoxic denitrification.

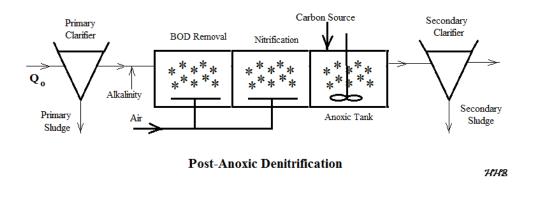
The pre-anoxic denitrification process has the advantage of not requiring an external carbon source and it reduces the BOD load to the BOD removal part of the process, because BOD is used in the denitrification reactions. However, the pre-anoxic process requires an influent C/N ratio greater than 4, where C/N is taken to be BOD/TKN, and the post-anoxic process can achieve a more complete nitrogen removal.

From the Odegaard references (#5 and #6 at the end of the course) suitable criteria for determining whether to use pre- or post-anoxic denitrification are as follows:

- Post-anoxic denitrification should be used if C/N < 4 or target % Removal of N > 75%

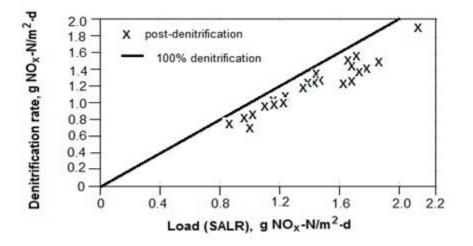
10. Post-Anoxic Denitrification Process Design Calculations

Process design of a post-anoxic denitrification MBBR system, requires sizing an MBBR tank for BOD removal, one for nitrification and one for denitrification. For all three of these reactors the key design parameter for sizing the MBBR tank is the surface area loading rate (SALR), typically with units of g/m²/day, that is g/day (of the parameter being removed in that reactor) coming into the MBBR tank per m² of carrier surface area in the reactor. Using design values for wastewater flow rate and concentration of the removed parameter entering the MBBR tank, the loading rate in g/day can be calculated. Then dividing the loading rate in g/day by the SALR in g/m²/day gives the required carrier surface area in m². The carrier fill %, carrier specific surface area, and carrier % void space can then be used to calculate the required carrier volume, tank volume and the volume of liquid in the reactor. A typical flow diagram for a post-anoxic denitrification MBBR process is shown in the figure on the next page.



MBBR Flow Diagram for Post-Anoxic Denitrification

Process design calculations for the BOD removal stage and the nitrification stage will be done just as described in **Section 8**, **Two-Stage BOD Removal and Nitrification Process Design Calculations**, and illustrated with the examples in that section. The process design calculations for denitrification, which is the third stage in a post-anoxic denitrification process, are similar to those previously discussed and illustrated for the BOD removal stage and the nitrification stage. The graph shown in the figure below (prepared using values from a similar graph in Rusten and Paulsrud's presentation in Ref #4 at the end of this course) will be used to obtain values for SARR/SALR vs SALR for the denitrification stage.



SARR vs SALR for Post-Anoxic Denitrification

Table 4 below shows typical SALR design values for pre-anoxic denitrification and postanoxic denitrification in MBBR reactors. Reference #2 at the end of this book is the source for the values in **Table 4**.

Typical Design Values for MBBR reactors at 15°C						
Purpose	Treatment Target % Removal	Design SALR g/m ² -d				
Denitrification Pre-DN (C/N > 4)	70 (NO ₃ -N)	0.90 (NO ₃ -N)				
Post-DN (C/N > 3)	90 (NO3-N)	2.00 (NO3-N)				

Table 4. Typical Design SALR Values for Denitrification

Example #7: A design wastewater flow of 1.5 MGD containing 175 mg/L BOD, 35 mg/L TKN, and alkalinity of 140 mg/L as CaCO₃ (in the primary effluent) is to be treated in a post-anoxic denitrification MBBR process. The design SALR for the first stage is to be 6 g BOD/m²/d. For the second stage, the SALR should be calculated for a target effluent NH₄-N conc. of 3.3 mg/L, min. design temperature of 45°F, and D.O. in reactor of 3.0 mg/L. For the post-anoxic stage, the SALR is to be 2 g NO₃-N/m²/d and the target effluent NO₃-N conc. is to be 5.0 mg/L.

For the third (Denitrification) stage calculate each of the following:

- i) The nitrate loading
- ii) The required carrier volume for a carrier with specific surface area of 600 m^2/m^3
- iii) The required MBBR tank volume for a design carrier fill % of 40%
- iv) The volume of liquid in the MBBR reactor for design carrier % void space of 60%.
- v) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
- vi) The estimated effluent NO₃-N concentration from the denitrification stage.
- vii) The alkalinity requirement in lb/day as $CaCO_3$ and in lb/day NaHCO₃, for target effluent alkalinity of 80 mg/L as $CaCO_3$.
- viii) The methanol requirement in lb/day for methanol use as the carbon source.

Note that the process design calculations for the BOD removal stage and the nitrification stage of this process will be the same as those used in Example #6 in the section on a two-stage BOD removal and nitrification process.

Solution - The solution is shown in the three figures on the next several pages, which are screenshots of different parts of an Excel worksheet used to carry out the calculations for this example. The first figure is from the top part of the worksheet and shows the user inputs and the calculation of the estimated **SARR/SALR** ratio for the

nitrification stage, resulting in a value of **SARR/SALR = 0.85**. The second figure is from the middle of the worksheet and shows the answers for parts i) through vi), as follows:

- i) The NO₃-N loading rate will be $(1.5 \text{ MGD})(35 3.3 \text{ mg/L})(8.34 \text{ lb/MG/mg/L}) = 396.6 \text{ lb/day} = (396.6 \text{ lb/day})*(453.59 \text{ g/lb}) = 179,879 \text{ g NO}_3-\text{N/day}$
- ii) Required carrier surface area = $(179,879 \text{ g/day})/(2 \text{ g/m}^2/\text{day}) = 89,939 \text{ m}^2$.

Required carrier volume = 89,939 m²/600 m²/m³. = **149.90 m³**

- iii) For 40% carrier fill: Required tank volume = $149.90 \text{ m}^3/0.40 = 374.75 \text{ m}^3$.
- iv) The volume of liquid in the reactor can be calculated as: tank volume – [carrier volume(1 – void %)], Thus the volume of liquid is: $374.75 - [149.90(1 - 0.60)] = 314.79 \text{ m}^3 = 314.79^*3.2808^3 = 11,117 \text{ ft}^3$.
- v) The HRT at design ave ww flow can be calculated as: $HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] =$ 11,117*7.48/[1.5*10⁶/(24*60)] = 80 min

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 80/4 = 20 min$

- vi) Calculation of the estimated effluent NO₃-N concentration from the second stage, as shown above for the BOD removal process, is shown in the screenshot below and gives a value of <u>4.8 mg/L</u>,
- vii) Calculation of the alkalinity requirement is shown in the third spreadsheet screenshot below. Using the equivalent weight of CaCO₃ as 50, the equivalent weight of NaHCO₃ as 84, the alkalinity use for nitrification as 7.14 g CaCO₃/g NH₃-N, the alkalinity produced by denitrification as 3.56 g CaCO₃/g NO₃-N denitrified, and the target effluent alkalinity as 80 mg/L as CaCO₃, give the calculated alkalinity requirement as 81.9 mg/L as CaCO₃, give the calculated alkalinity requirement as 81.9 mg/L as CaCO₃. The rate of alkalinity addition needed can then be calculated as: (1.5 MGD)(81.9 mg/L)*8.34 = <u>1024.9 lb/day as CaCO₃</u>. Multiplying this by the ratio of the equivalent weight of NaHCO₃ to the equivalent weight of CaCO₃ gives the daily NaHCO₃ requirement as <u>1721.8 lb/day NaHCO₃</u>.
- viii) Calculation of the methanol requirement in lb/day is shown at the bottom of the third screenshot below. As shown, the calculations use the constants, 4.6 lb COD/lb NO₃-N removed and 1.5 lb COD/lb Methanol. The required methanol dosage is then calculated as: 4.6/1.5 = 3.1 lb methanol /lb NO₃-N removed. The methanol requirement in lb/day is then equal to 3.1 times the previously calculated NO₃-N removal rate of 337.1 lb/day, or **1033.7 lb/day**.

Instructions: Enter value	es in blue	boxes. S	Spreadsheet calcula	ates values	in yellow b	oxes
I. Wastewater Parameter						
. Waste water i arameter	mputs				-	
1. Parameters for All Three	e Stages					
Design ww Flow Rate, Q _o =	1.5	MGD	Prim. Effl. B	2000-00-00-00-00-00-00-00-00-00-00-00-00	175	mg/L
Prim. Effl. TKN Conc. =	35	mg/L	Prim. Effl. NO3	3-N Conc. =	0	mg/L
Peak Hour Factor =	4	Pri	m. Effl. Alkalinity =	140	mg/L as C	aCO3
2. Parameters for First (BC	DD Remo	val) Stag	e:			
		1.1-1-1.1	Data points for	SARR/SALR	vs SALR	
Design Value of BOD Surface			SALR (g/m²/d):	7.5	15.0
Area Loading Rate (SALR) =	6	g/m²/d	SARR/	SALR:	0.925	0.875
See information on typical d	esign		(default values abov			
values for SALR at right.			of % BOD removal	vs SALR show	wn at the rig	ht from ref
Est. of SARR/SALR Rato =	0.935					
(Surf. Area Removal Rate/Sur	f. Area Loa	ding Rate)	Slope, S	ARR/SALR vs	SALR:	-0.007
			Intercept, S	ARR/SALR vs	SALR:	0.975
3. Parameters for Second		ion stay	e.			
Influent NH ₄ -N Conc. =	25	mg/L	Target Effl NH₄-N Cond	c, NH ₄ -N _e =	3.3	mg/L
Influent NH ₄ -N Conc. =	25	mg/L	Target Effl NH4-N Conc Click on cell H26 a		to select D.	
Influent NH ₄ -N Conc. =	25 45	mg/L °F	Target Effl NH ₄ -N Cond Click on cell H26 a D.O Cond	nd on arrow	to select D.	O Conc.
Influent NH ₄ -N Conc. = Min Design Temp., T = <u>4. Preliminary Calculatio</u>	25 45 9 ns - Des	mg/L °F	Target Effl NH ₄ -N Cond Click on cell H26 a D.O Cond fication SALR valu	nd on arrow :. in Reactor = Je	to select D.(O Conc. mg/L
Influent NH ₄ -N Conc. = Min Design Temp., T =	25 45	mg/L °F ign Nitrit	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond Fication SALR valu NH₄-N _€ @ SA	nd on arrow :. in Reactor = <u>Je</u> RR _{max} =	to select D.(3.0 0.80	O Conc. mg/L mg/L
Influent NH ₄ -N Conc. = Min Design Temp., T = <u>4. Preliminary Calculatio</u>	25 45 9 ns - Des	mg/L °F ign Nitrit	Target Effl NH ₄ -N Cond Click on cell H26 a D.O Cond fication SALR valu NH ₄ -N _e @ SA SARR @ NH ₄ -Ne, 15 ^o	nd on arrow in Reactor = 	to select D.(3.0 0.80	O Conc. mg/L mg/L g/m²/d
Influent NH ₄ -N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH ₄ -N removal = Maximum SARR =	25 45 ons - Des 87%	mg/L °F ign Nitrit g/m²/d	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond fication SALR valu NH₄-N∉ @ SA SARR @ NH₄-Ne, 15 ⁰ SARR @ NH₄-Ne, 1 °C	nd on arrow c. in Reactor = <u>Je</u> RR _{max} = C, SARR ₁₅ C, SARR ₇ =	 to select D. 3.0 0.80 0.88 0.57 	O Conc. mg/L mg/L g/m²/d g/m²/d
Influent NH ₄ -N Conc. = Min Design Temp., T = <u>4. Preliminary Calculatio</u> % NH ₄ -N removal =	25 45 ons - Des 87%	mg/L °F ign Nitrit g/m²/d	Target Effl NH ₄ -N Cond Click on cell H26 a D.O Cond fication SALR valu NH ₄ -N _e @ SA SARR @ NH ₄ -Ne, 15 ^o	nd on arrow c. in Reactor = <u>Je</u> RR _{max} = C, SARR ₁₅ C, SARR ₇ =	 to select D. 3.0 0.80 0.88 0.57 	O Conc. mg/L mg/L g/m²/d
Influent NH ₄ -N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH ₄ -N removal = Maximum SARR =	25 45 ons - Des 87% 0.88	mg/L °F ign Nitrit g/m²/d	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond fication SALR valu NH₄-N∉ @ SA SARR @ NH₄-Ne, 15 ⁰ SARR @ NH₄-Ne, 1 °C	nd on arrow c. in Reactor = <u>Je</u> RR _{max} = C, SARR ₁₅ C, SARR ₇ =	 to select D. 3.0 0.80 0.88 0.57 	O Conc. mg/L mg/L g/m²/d g/m²/d
Influent NH ₄ -N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH ₄ -N removal = Maximum SARR =	25 45 ons - Des 87% 0.88 1.058	mg/L °F ign Nitrit g/m²/d Di	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond fication SALR valu NH₄-N∉ @ SA SARR @ NH₄-Ne, 15% SARR @ NH₄-Ne, 15% SARR @ NH₄-Ne, T % esign Value for nitrificat	nd on arrow c. in Reactor = <u>Je</u> RR _{max} = C, SARR ₁₅ C, SARR ₇ =	 to select D. 3.0 0.80 0.88 0.57 	O Conc. mg/L mg/L g/m²/d g/m²/d
Influent NH ₄ -N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH ₄ -N removal = Maximum SARR = SARR Temp. Coeff, θ = 5. Parameters for Third (P	25 45 ons - Des 87% 0.88 1.058 ost-Anoxi	mg/L °F ign Nitrii g/m²/d Di ic) Stage	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond fication SALR valu NH₄-N₅ @ SA SARR @ NH₄-Nε, T°C SARR @ NH₄-Ne, T°C esign Value for nitrificat Data points for	IN CONTRACTOR SARR/SALR	 to select D. 3.0 0.80 0.88 0.57 0.65 	O Conc. mg/L g/m²/d g/m²/d g/m²/d
Influent NH ₄ -N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH ₄ -N removal = Maximum SARR = SARR Temp. Coeff, θ =	25 45 ons - Des 87% 0.88 1.058	mg/L °F ign Nitrit g/m²/d Di	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond fication SALR valu NH₄-N₅ @ SA SARR @ NH₄-Nε, T°C SARR @ NH₄-Ne, T°C esign Value for nitrificat Data points for	nd on arrow in Reactor = Je RR _{max} = C, SARR ₁₅ C, SARR ₇ = ion SALR =	 to select D. 3.0 0.80 0.88 0.57 0.65 	O Conc. mg/L mg/L g/m²/d g/m²/d
Influent NH ₄ -N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH ₄ -N removal = Maximum SARR = SARR Temp. Coeff, θ = 5. Parameters for Third (P	25 45 ons - Des 87% 0.88 1.058 ost-Anoxi	mg/L °F ign Nitrii g/m²/d Di ic) Stage mg/L	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond Fication SALR valu NH₄-N∉ @ SA SARR @ NH₄-Ne, 15% SARR @ NH₄-Ne, 15% SARR @ NH₄-Ne, 1 % esign Value for nitrificat Data points for SALR (In Reactor = IE RR _{max} = C, SARR ₁₅ = C, SARR _T = ion SALR = SARR/SALR	 to select D. 3.0 0.80 0.88 0.57 0.65 vs SALR 	O Conc. mg/L g/m²/d g/m²/d g/m²/d
Influent NH ₄ -N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH ₄ -N removal = Maximum SARR = SARR Temp. Coeff, θ = 5. Parameters for Third (P Target Effl. NO ₃ -N Conc. =	25 45 ons - Des 87% 0.88 1.058 ost-Anoxi	mg/L °F ign Nitrii g/m²/d Di ic) Stage	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond Fication SALR valu NH₄-N∉ @ SA SARR @ NH₄-Ne, 15% SARR @ NH₄-Ne, 15% SARR @ NH₄-Ne, 1 % esign Value for nitrificat Data points for SALR (Ind on arrow in Reactor = IE RR _{max} = C, SARR ₁₅ C, SARR ₇ = ion SALR = SARR/SALR g/m ² /d): SALR:	 to select D. 3.0 0.80 0.88 0.57 0.65 vs SALR 1 0.880 	O Conc. mg/L g/m ² /d g/m ² /d g/m ² /d 2.0 0.850
Influent NH₄-N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH₄-N removal = Maximum SARR = SARR Temp. Coeff, θ = 5. Parameters for Third (P Target Effl. NO ₃ -N Conc. = Design Value of BOD Surface Area Loading Rate (SALR) = See information on typical do	25 45 ons - Des 87% 0.88 1.058 ost-Anoxi 5.0 2.0	mg/L °F ign Nitrii g/m²/d Di ic) Stage mg/L	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond fication SALR valu NH₄-N∉ @ SA SARR @ NH₄-Ne, 15 ^o SARR @ NH₄-Ne, 1 5 ^o SARR @ NH₄-Ne, 1 °C esign Value for nitrificat Data points for SALR (SARR/ (default value Post-Anoxic S	Ind on arrow in Reactor = Je RR _{max} = C, SARR ₁₅ = C, SARR ₇ = ion SALR = SARR/SALR g/m ² /d): SALR: s above are SARR vs SAL	<pre>to select D.(</pre>	0 Conc. mg/L g/m ² /d g/m ² /d g/m ² /d 2.0 0.850 h of
Influent NH₄-N Conc. = Min Design Temp., T = 4. Preliminary Calculation % NH₄-N removal = Maximum SARR = SARR Temp. Coeff, θ = 5. Parameters for Third (P Target Effl. NO ₃ -N Conc. = Design Value of BOD Surface Area Loading Rate (SALR) =	25 45 ons - Des 87% 0.88 1.058 ost-Anoxi 5.0 2.0	mg/L °F ign Nitrii g/m²/d Di ic) Stage mg/L	Target Effl NH₄-N Cond Click on cell H26 a D.O Cond fication SALR valu NH₄-N∉ @ SA SARR @ NH₄-Ne, 15 ^o SARR @ NH₄-Ne, 1 5 ^o SARR @ NH₄-Ne, 1 °C esign Value for nitrificat Data points for SALR (SARR/ (default value Post-Anoxic S	Ind on arrow in Reactor = IE RR _{max} = C, SARR ₁₅ C, SARR ₇ = ion SALR = SARR/SALR g/m ² /d): SALR: s above are	<pre>to select D.(</pre>	0 Conc. mg/L g/m ² /d g/m ² /d g/m ² /d 2.0 0.850 h of
Influent NH₄-N Conc. = Min Design Temp., T = 4. Preliminary Calculatio % NH₄-N removal = Maximum SARR = SARR Temp. Coeff, θ = 5. Parameters for Third (P Target Effl. NO ₃ -N Conc. = Design Value of BOD Surface Area Loading Rate (SALR) = See information on typical de	25 45 ons - Des 87% 0.88 1.058 ost-Anoxi 5.0 2.0	mg/L °F ign Nitrii g/m²/d Di ic) Stage mg/L	Target Effl NH4-N Cond Click on cell H26 a D.O Cond fication SALR valu NH4-Ne @ SA SARR @ NH4-Ne, 15% SARR @ NH4-Ne, T % esign Value for nitrificat Data points for SALR (SARR/ (default value Post-Anoxic S (The graph i	Ind on arrow in Reactor = Je RR _{max} = C, SARR ₁₅ = C, SARR ₇ = ion SALR = SARR/SALR g/m ² /d): SALR: s above are SARR vs SAL	to select D.(3.0 0.80 0.88 0.57 0.65 vs SALR 1 0.880 from a grapl R in ref #6 b he right.)	0 Conc. mg/L g/m ² /d g/m ² /d g/m ² /d 2.0 0.850 h of

Screenshot – Post-Anoxic Denitrification Design Calculations – Part 1

Carbon Nitragon Datia	5.0	3	1100	this Werksheat for Deet (and	
Carbon:Nitrogen Ratio, C/N =	5.0			this Worksheet for Post-A	ANOXIC	
Target % N removal =	86%		Deni	trification calculations		
III. Carrier and Tank Sha	pe Para	meter Inj	outs fo	or all Three Stages		
Carrier Spec. Surf. Area =	600	m ² /m ³	Click	on green box and then on		
(from carrier mfr/vendor)			arro	w to Select Tank Shape:	rectar	ngular
Liquid Depth in Tank =	8	ft		Carrier % Void Space =	60%	
Tank L:W ratio =	1.5			(from carrier mfr/vendor -	only needed	to
(target L:W - only used if ta	nk is recta	ngular)		calculate hydraulic deter	ntion time)	
3. Third Stage (Post-Anox			quire	d Tank Volume & Dimer	nsions	
				d Tank Volume & Dimer Required Tank Volume =	472.86	m ³
3. Third Stage (Post-Anox	ic) Calcu	lations_				m ³ ft ³
3. Third Stage (Post-Anox	ic) Calcu 32%	lations (for third :		Required Tank Volume =	472.86	
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading =	ic) Calcu 32% <u>396.6</u>	lations (for third : Ib/day		Required Tank Volume =	472.86 16699.0	ft ³
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading =	ic) Calcu 32% 396.6 179879	(for third solutions) (for third solution) (for third solution) (for third solution)		Required Tank Volume = Tank Liquid Volume =	472.86 16699.0 124909	ft ³ gal
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed =	ic) Calcu 32% 396.6 179879 89939	(for third a lb/day g/day m ²	stage)	Required Tank Volume = Tank Liquid Volume = Calculated Tank Width =	472.86 16699.0 124909 37.3 56.0	ft ³ gal ft
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar	ic) Calcu 32% 396.6 179879 89939 149.90 14581.6	(for third a lb/day g/day m ² m ³	stage)	Required Tank Volume = Tank Liquid Volume = Calculated Tank Width = Calculated Tank Length =	472.86 16699.0 124909 37.3 56.0	ft ³ gal ft
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar Removal Rate, SARR =	ic) Calcu 32% 396.6 179879 89939 149.90 14581.6	(for third a lb/day g/day m ² m ³	stage)	Required Tank Volume = Tank Liquid Volume = Calculated Tank Width = Calculated Tank Length = minal Hydraulic Retention Time	472.86 16699.0 124909 37.3 56.0 e at	ft ³ gal ft ft
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar	ic) Calcu 32% 396.6 179879 89939 149.90 14581.6 ea	(for third s lb/day g/day m ² m ³ ft ³	stage) Nor	Required Tank Volume = Tank Liquid Volume = Calculated Tank Width = Calculated Tank Length = minal Hydraulic Retention Time Design Average Flow =	472.86 16699.0 124909 37.3 56.0 e at 105	ft ³ gal ft ft min
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar Removal Rate, SARR =	ic) Calcu 32% 396.6 179879 89939 149.90 14581.6 ea 1.70	lations (for third a lb/day g/day m ² m ³ ft ³ g/m ² /d	stage) Nor	Required Tank Volume = Tank Liquid Volume = Calculated Tank Width = Calculated Tank Length = minal Hydraulic Retention Time Design Average Flow = Peak Hourly Flow =	472.86 16699.0 124909 37.3 56.0 e at 105 26 4.8	ft ³ gal ft ft min min
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar Removal Rate, SARR =	ic) Calcu 32% 396.6 179879 89939 149.90 14581.6 ea 1.70 152897	lations (for third lb/day g/day m ² m ³ ft ³ g/m ² /d g/day	stage) Nor	Required Tank Volume = Tank Liquid Volume = Calculated Tank Width = Calculated Tank Length = minal Hydraulic Retention Time Design Average Flow = Peak Hourly Flow = Calculated Effl NO ₃ -N Conc.:	472.86 16699.0 124909 37.3 56.0 e at 105 26 4.8 -N conc. Is	ft ³ gal ft ft min min
3. Third Stage (Post-Anox Design Carrier Fill % = NO ₃ -N Daily Loading = NO ₃ -N Daily Loading = Carrier Surf. Area needed = Calculated Carrier Volume = Tank Liquid Volume = Estimate of NO ₃ -N Surface Ar Removal Rate, SARR = Est. of NO ₃ -N Removal Rate:	ic) Calcu 32% 396.6 179879 89939 149.90 14581.6 ea 1.70 152897	lations (for third lb/day g/day m ² m ³ ft ³ g/m ² /d g/day	stage) Nor	Required Tank Volume = Tank Liquid Volume = Calculated Tank Width = Calculated Tank Length = minal Hydraulic Retention Time Design Average Flow = Peak Hourly Flow = Calculated Effl NO ₃ -N Conc.: If the calculated Effl. NO ₃	472.86 16699.0 124909 37.3 56.0 e at 105 26 4.8 -N conc. Is ue of SALR	ft ³ gal ft ft min min

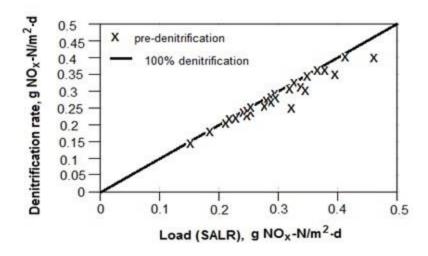
Screenshot – Post-Anoxic-Denitrification Design Calculations – Part 2

Input:	Taro	et Effluent	Alkalinity =		80	mg/L as CaC	O ₃	
Constants	needed for Ca	lculations	<u>x</u>			-		
Equiv Wi	t. of CaCO ₃ =	50	g/equiv.		Equiv Wt.	of NaHCO ₃ =	84	g/equiv
Alkalini	ity used for Nitrifica	ation =	7.14	g CaC	CO ₃ /g NH ₃ -N re	emoved		
Alkalinity	produced by Denit	rification =	3.57	g CaO	CO ₃ /g NO ₃ -N r	emoved		
Calculatio	ns							
	Alkalinity to be ad	ded =	81.9	mg/L	as CaCO3			
Daily	y Alkalinity Require	ement =	1024.9	lb/da	y as CaCO ₃			
For sodi	ium bicarbonate	use to add	alkalinity:					
Daily	y NaHCO ₃ Require	ment =	1721.8	lb/da	y NaHCO₃			
VII. Calcu	lation of Carb	on Sourc	e Require	emen	ts			
Inputs:	C	arbon Sourc	e to be use	d:	Methanol			
	COD Requireme	ent for Denit	rification =		4.6	Ib COD/Ib NO3	-N removed	
	COD Conte	nt of Carbor	Source =		1.5	Ib COD/Ib Carl	bon Source	
Calculatio	ins ins							
	Carbon Source Do	sage =	3.1	Ib Ca	rbon Source/Ib	NO ₃ -N removed		
Daily Ca	rbon Source Requi	rement =	1033.7	lb/da	v			

Screenshot – Post-Anoxic Denitrification Design Calculations – Part 3

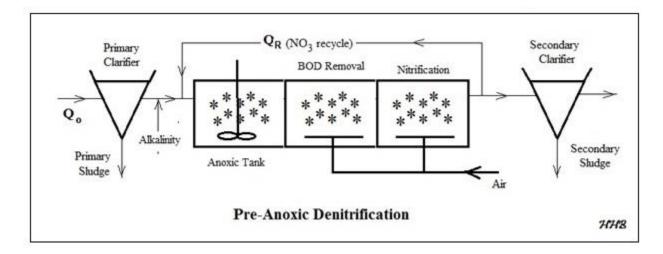
11. Pre-Anoxic Denitrification Process Design Calculations

The process design calculations for pre-anoxic denitrification, are similar to those just discussed for a post-anoxic denitrification process. The graph shown in the figure below (prepared using values from a similar graph in Rusten and Paulsrud's presentation in Ref #4 at the end of this course) will be used to obtain values for SARR/SALR vs SALR for the pre-anoxic denitrification stage.



SARR vs SALR for Pre-Anoxic Denitrification

A typical flow diagram for a pre-anoxic denitrification MBBR process is shown in the figure below. As discussed previously for a post-anoxic denitrification MBBR system, process design of a pre-anoxic denitrification MBBR system also requires sizing an MBBR tank for BOD removal, one for nitrification and one for denitrification. Process design for the nitrification stage is essentially the same as just discussed for the post-anoxic denitrification process.



MBBR Flow Diagram for Pre-Anoxic Denitrification

Process Design calculations for the BOD removal stage will be a bit different from those for the BOD removal stage in the post-anoxic process, because some of the incoming BOD is used as the carbon source for the denitrification reactions in the pre-anoxic tank. For the pre-anoxic process, the BOD loading rate (in lb/day) should be calculated as:

BOD Daily Loading = $(Q_0 * S_0 * 8.34) - (0.67*(20/7)*NO_3 - N removal rate)$

The second term is the estimated BOD removal rate in the anoxic reactor through its use in the denitrification reactions. This gives a lower BOD loading rate than that for the post-anoxic process with the same primary clarifier effluent coming in. Hence, the required tank size for BOD removal will be smaller for the pre-anoxic process.

The main difference from the post-anoxic denitrification process design calculations is for the denitrification stage, which will be discussed and illustrated with **Example #8** calculations below.

Example #8: Carry out the process design as described below for the denitrification stage of a pre-anoxic denitrification process with the wastewater flow and concentrations given in **Example #7**. [1.5 MGD containing 175 mg/L BOD and 35 mg/L TKN (in the primary effluent)]. Consider that the primary effluent alkalinity is 140 mg/L as CaCO₃ and the design SALR for the denitrification stage is to be 0.9 g NO₃-N/m²/d. The nitrification stage was designed for an effluent NH₃-N concentration of 3.3 mg/L, a D.O. of 3.0 mg/L, and a minimum WW temperature of 45°F.

For the first stage (denitrification) calculate each of the following:

- i) The nitrate loading rate
- ii) The required carrier volume for a carrier with specific surface area of 600 m^2/m^3
- iii) The required MBBR tank volume for a design carrier fill % of 40%
- iv) The volume of liquid in the MBBR reactor for design carrier % void space of 60%.
- v) The average hydraulic retention time at design average wastewater flow and at design peak hourly flow if the design peak hour factor is 4.
- vi) The required NO₃-N recycle rate needed to achieve a target effluent NO₃-N concentration of 9 mg/L.
- vii) The alkalinity requirement in lb/day as CaCO₃ and in lb/day NaHCO₃, for a target effluent alkalinity of 80 mg/L as CaCO₃.

Solution - The solution is shown in the three figures on the next several pages, which are screenshots of different parts of an Excel worksheet used to carry out the calculations for this example. The first figure below is from the top part of the worksheet and shows the user inputs and the calculation of the estimated **SARR/SALR** ratio for the denitrification stage (calculated to be **0.927)**. Note also that a user input value is needed for the estimated NO₃-N recycle ratio. This initial estimated value is used in an iterative calculation farther down on the worksheet (in the second screenshot below) to

determine the required NO₃-N recycle ratio needed to achieve the target effluent NO₃-N concentration.

Instructions: Enter value	s in blue	boxes. S	preadsheet calculat	es values in	yellow box	es
I. Wastewater Parameter	Inputs					
1. Parameters for All Three	Stages					
Design ww Flow Rate, Qo =	1.5	MGD	Prim. Effl. B	OD, S _o =	175	mg/L
Prim. Effl, TKN Conc. =	35	mg/L	Prim. Effl. NO	3-N Conc. =	0	mg/L
Peak Hour Factor =	4	Pr	im. Effl. Alkalinity =	140	mg/L as Ca	ICO3
2. Parameters for First (Pr	e-Anoxic)	Stage:				
			Data points for			
Target Effl. NO ₃ -N Conc. =	9	mg/L	SALK	g/m²/d):	0.2	0.5
Est. of NO ₃ -N Recycle Rato:	2.72		SARR	SALR:	0.95	0.94
(Q _R /Q _o) - An estimate is need	ded here t	o start	(default values above	are from a	graph of Pre-	Anoxic
the iterative calculation in S	ec IV belo	w	SARR vs SALR in	ref #6 below	right)	
Design Value of NO3-N Surface			(The graph is sh			
Area Loading Rate (SALR) =	0.9	g/m²/d	Slope, S	ARR/SALR vs	SALR:	-0.033
See information on typical de	sign		Intercept, S	ARR/SALR vs	SALR:	0.957
values for SALR at right.			Est	of SARR/SAL	R Rato =	0.927
			(Surf. Area Removal	Rate/Surf Are	a Loading Rat	(e)

The figure, on the next page, is from the middle of the worksheet and shows the answers for parts i) through vi). The calculations and results are as follows:

Most of the nitrate loading to the pre-anoxic denitrification tank is typically in the i) NO₃-N recycle flow rather than in the primary effluent flow entering the tank. The NO₃-N loading rate will be: (1.5 MGD)(Prim Effl NO₃-N)(8.34 lb/MG/mg/L) + (1.5 MGD)(Recycle Ratio)(Target Effl NO₃-N)(8.34) = 306.5 lb/day

 $= (306.5 \text{ lb/day})^*(453.59 \text{ g/lb}) = 139,003 \text{ g NO}_3 \text{-N/day}$

ii) Required carrier surface area = $(139,003 \text{ g/day})/(0.9 \text{ g/m}^2/\text{day}) = 154,447 \text{ m}^2$.

Required carrier volume = $154,447 \text{ m}^2/600 \text{ m}^2/\text{m}^3$. = 257.41 m^3 = $(257.41 \text{ m}^3)(3.2808^3 \text{ ft}^3/\text{m}^3) = 9090 \text{ ft}^3.$

iii) For 40% carrier fill: Required tank volume = 9090 ft³/0.40 = $\frac{22,726 \text{ ft}^3}{(643.5 \text{ m}^3)}$

Carbon:Nitrogen Ratio, C/N =	5.0		llest	his Worksheet for P		via		-
	5.0	-		18 0. INTR. 18 19 19 19 19	-	XIC		-
Target % N removal =	74%		Denit	rification calculation	ns	-		2
		2 2	1					2
III. Carrier and Tank Sha	pe Paran	neter Inp	uts fo	r all Three Stages				
Carrier Spec. Surf. Area =	600	m ² /m ³	Click	on green box and the	n on			
(from carrier mfr/vendor)			12202252/25	v to Select Tank Shar		rectar	ngular	
_iquid Depth in Tank =	8	ft		Carrier % Void Spac		60%		
Tank L:W ratio =	1.5			(from carrier mfr/ve	19915 - 19	nly needed	to	
target L:W - only used if tan	k is rectanç	jular)		calculate hydrauli	c deten	tion time)		
V. Calculation of Carrie	r Volume	and Rec	uired	Tank Volume & D	imensi	ons		
1. First Stage (Pre-Anoxic	Tank) Cal	culations	5	(Carrier fill % is typic				
		_		values are more con			Contraction of the second	100252
Design Carrier Fill % =	40%	(for first s	tage)	expansion or reduct	ion of S	ALR by add	120	arrier
NO ₃ -N Daily Loading =	306.5	lb/day		Calculated Tank Volu	me =	643.5	m ³	
NO ₃ -N Daily Loading =	139002.7	g/day				22726.1	ft ³	
Carrier Surf. Area needed =	154447.4	m ²				169991	gal	
Calculated Carrier Volume =	257.412	m ³		Calculated Tank Wid	lth =	43.5	ft	
Tank Liquid Volume =	19089.9	ft ³		Calculated Tank Len	gth =	65.3	ft	
			Non	ninal Hydraulic Retentio	n Time <mark>a</mark>	at		
Estimate of NO ₃ -N Surface Ar	ea			Design Average Flow	/ =	137	min	
Removal Rate, SARR =	0.83	g/m²/d		Peak Hourly Flow	=	34	min	
Req'd NO ₃ -N Removal Rate:	283.98	lb/day	E	st. of NO ₃ -N Removal R	ate:	283.98	g/day	
(for Effl. NO ₃ -N = Target Valu	ie)		E	st. Rem Rate - Req'd Ra	ate:	0.0000	g/day	
- 11		1						
NOTE: Use Excel's "Goal								
"Goal Seek" (in the "tools"								
versions of Excel). Enter va								
		10 10		ear in cell C17 and ce	11 H73	chould oou	alzoro	

iv) The volume of liquid in the reactor can be calculated as:

tank volume – [carrier volume(1 – void %)]. Thus, the volume of liquid is: $22,726 - [9090(1 - 0.60)] = 19,090 \text{ ft}^3$. (540.6 m³)

v) The HRT at design ave WW flow can be calculated as: $HRT_{des ave} = reactor liquid volume*7.48/[Q*10⁶/(24*60)] = 19,090*7.48/[1.5*10⁶/(24*60)] = 137 min$

 $HRT_{peak hr} = HRT_{des ave}/peak hour factor = 137/4 = 34 min$

vi) The required NO₃-N recycle ratio is calculated with the iterative process described in blue at the bottom of the last figure above. For this iterative process, the NO₃-N removal rate is calculated two different ways, one using the estimated SARR and the carrier surface area while the other uses the wastewater flow rate times the influent TKN concentration minus the sum of the effluent nitrate and ammonia nitrogen concentrations. Excel's Goal Seek process is then used to set the difference between the two different calculations equal to zero by changing the estimated value of the NO₃-N recycle ratio. This process results in the required NO₃-N recycle ratio calculated to be **2.72**.

vii) Calculation of the alkalinity requirement is shown in the third screenshot figure, below on this page. Using the equivalent weight of CaCO₃ as 50, the equivalent weight of NaHCO₃ as 84, the alkalinity use for nitrification as 7.14 g CaCO₃/g NH₃-N, the alkalinity produced by denitrification as 3.56 g CaCO₃/g NO₃-H, and the target effluent alkalinity as 80 mg/L as CaCO₃, give the calculated alkalinity requirement as **97.1 mg/L as CaCO₃**. The rate of alkalinity addition needed can then be calculated as: (1.5 MGD)(91.1 mg/L)*8.34 = <u>1214.5 lb/day as CaCO₃</u>. Multiplying this by the ratio of the equivalent weight of NaHCO₃ to the equivalent weight of CaCO₃ gives the daily NaHCO₃ requirement as <u>2040.3 lb/day NaHCO₃</u>.

VII. Calculation	n of Alkalin	ity Requ	irements	-			
Input:	Targ	get Effluent	Alkalinity =	80	mg/L		
Constants need	led for Calo	ulations					
Equiv Wt. of Ca	CO3 =	50	g/equiv.	Equiv Wt. (of NaHCO ₃ =	84	g/equiv.
Alkalinity used	d for Nitrificati	on =	7.14	g CaCO ₃ /g NH ₃ -N rer	noved		
Alkalinity produc	ed <mark>b</mark> y Denitrif	ication =	3.57	g CaCO ₃ /g NO ₃ -N rei	moved		
Calculations							
Alkalir	ity to be add	ed =	97.1	mg/L as CaCO3			
Daily Alkalinity Requirement =		1214.5	lb/day as CaCO ₃				
For sodium bi	carbonate u	se to add a	alkalinity:		1		
Daily NaHC	O ₃ Requirem	ent =	2040.3	Ib/day NaHCO ₃			

12. Summary

The MBBR (moving bed biofilm reactor) process is an attached growth process that uses plastic carriers to provide a surface on which biofilm grows. The plastic carriers are kept suspended in the aeration tank by an aerator for an aerobic process or by mechanical mixing for an anoxic or anaerobic process. The plastic carriers are kept in the system by a sieve at the outlet of the tank. The MBBR process doesn't require sludge recycle, because the biomass remains in the system attached to the plastic carriers. The required reactor size for an MBBR process is typically significantly smaller than that for an activated sludge process treating the same wastewater flow, or for other common attached growth processes like the RBC or trickling filter. It can be used for BOD removal, biological nitrification, biological denitrification, and biological phosphorus removal.

This course provides discussion of and detailed examples of process design calculations for a single stage BOD removal MBBR system, a two-stage BOD removal MBBR system, a single stage tertiary nitrification MBBR system, a two-stage BOD removal/Nitrification MBBR system a post-anoxic denitrification MBBR system and a pre-anoxic denitrification MBBR system.

13. References

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